1 2	3	4 5	6
COORDINATION	CODES & REFERENCED REPORTS	DESIGN LOADS CONTINUED	DRILLED PIERS
A. The Contractor shall compare the Architectural, Structural, Mechanical, Electrical, Plumbing, and other series drawings and report any discrepancies between each set of drawings and within each set of drawings prior to fabrication and installation of any	<ul> <li>A. The General Building Code used as the basis for the structural design is as follows:</li> <li>1. International Building Code. 2015 Edition with City of Austin amendments</li> </ul>	<ul> <li>Pressures for Tributary Areas in between the listed values may be linearly interpolated.</li> <li>Negative value signifies pressure acting away from the surface (suction)</li> </ul>	A. Due to the topography of the site, a geotech time and will be completed once clearing op of the structure is based on preliminary reco
structural members. B. Only larger sleeve openings and framed openings in structural framing component	<ul> <li>B. Structural Concrete: Building Code Requirements for Reinforced Concrete, American Concrete Institute, ACI 318, as referenced by the General Building Code.</li> </ul>	<ul> <li>Edge and Corner zone distances shall be determined in accordance with referenced standard.</li> <li>* Pressures are for gross uplift conditions. Refer to roof plan(s) for net uplift</li> </ul>	geotechnical engineer, based on their experi investigation will be used to confirm the pre necessary, the design of the foundation and
members are indicated on the Structural Drawings. However, all sleeves, inserts and openings, including frames and/or sleeves shall be provided for passage, provision and/or incorporation of the work of the contract, including but not limited to	C. Concrete Masonry: Building Code Requirements for Concrete Masonry Structures,	values for design of joists, joist girders, and bridging.	the actual values. B Pier design is based on the following design
Mechanical, Electrical and Plumbing work. This work shall include the coordination of sizes, alignment, dimensions, position, locations, elevations and grades as required to serve the intended purpose. Openings not indicated on the Structural Drawings, but required as noted above, shall be submitted to the Engineer for review.	<ul> <li>D. Structural Steel: Manual of Steel Construction, American Institute of Steel</li> <li>Construction Inc., ANSI/AISC 360, as referenced by the General Building Code.</li> </ul>	<ol> <li>The structure and structural components of the enclosure have been designed in accordance with General Building Code with the following criteria for Austin, Texas per IBC 2015:</li> </ol>	<ul> <li>1. Side friction: 4 ksf</li> <li>C. Pier design is in accordance with the recommon report.</li> </ul>
C. Refer to Architectural, Mechanical, Electrical and Plumbing drawings for floor elevations, slopes, drains and location of depressed and elevated floor areas.	E. Geotechnical Report: Foundation elements have been designed in accordance with information provided in the following geotechnical report:	<ul> <li>a. Seismic Importance Factor, IE</li> <li>b. Risk Category</li> <li>c. Mapped Spectral Response Accelerations</li> </ul>	D. Bearing stratum shown on the pier details is
D. Compatibility of the structure and provisions for enclosure equipment supported on or from structural components shall be verified as to size, dimensions, clearances, accessibility, weights and reaction with the equipment for which the structure has	Geotechnical Engineer: HDR Report Number: 10123906 Date: 07-08-2020	i. Ss (%g) ii. S1 (%g) d. Site Class e. Spectral Besponse Coefficients 0.063 0.033 D (assumed)	<ul> <li>E. Piers not specifically located on the plan sha above. Where no column occurs, locate on c</li> <li>E. Provide dowels from piers into concrete above.</li> </ul>
been designed prior to submission of shop drawings and data for each piece of equipment and for structural components. Differences shall be noted on the submittals.		i. SDS 0.067 ii. SD1 0.052 f. Seismic Design Category A	shown for pilaster/column/wall above. Whe and number as pier reinforcing steel. Extenc as shown on the Structural Drawings. Exten
E. The details designated as "Typical Details" apply generally to the Structural Drawings in all areas where conditions are similar to those described in the details.		g. Basic Seismic-force-resisting system: Ordinary Reinforced Masonry Shear Walls	G. Elevation of top of piers, unless noted otherwise on the
F. All structural elements of the project have been designed by the Engineer to resist the required code vertical and lateral forces that could occur in the final completed	DESIGN LOADS	h. Design Base Shear, V 0.01W, W=Effective seismic weight of structure	bottom of the deepest intersecting beam or H. Reinforcing cage shall be held securely away
structure only. It is the responsibility of the Contractor to provide all required bracing during construction to maintain the stability and safety of all structural elements during the construction process until the lateral-load resisting or stability-providing	<ul> <li>A. Dead Loads include the self-weight of the structural elements and the following superimposed loads:</li> <li>1. Calling and Machanical et react.</li> </ul>	<ul> <li>i. Seismic Response Coefficient(s), Cs</li> <li>j. Response Modification Factor(s), R</li> <li>k. Analysis Procedure Used</li> <li>Equivalent Lateral Force</li> </ul>	3 spacers at a maximum spacing of 8 ft. along the bottom.
system is completely installed and the structure is completely tied together. Temporary supports shall not result in the overstress or damage of the elements to be braced nor any elements used as brace supports.	B. Live Loads	Procedure F. Balcony Railing and Guardrails: The balcony railings and guardrails shall be designed	<ol> <li>Pier reinforcing and concrete shall be placed complete; in no case shall a pier be drilled th workday.</li> </ol>
G. The Contract Structural Drawings and Specifications represent the finished structure, and except where specifically shown, do not indicate the means or methods of	OCCUPANCY OR USE UNIFORM CONCENTRATED (psf) (lbs.)	for 50 pounds/ft load applied horizontally at right angles to the top rail or a 200 pound concentrated load applied in any direction at any point along the top rail, whichever is greater. The railing shall have attachment devices to adequately anchor to the	J. See plans for pier sizes, reinforcing and dept
construction. The Contractor and their Sub-Contractors shall supervise and direct the Work and shall be solely responsible for all construction means, methods, procedures, techniques, sequences and safety measures including, but not limited to, adherences	<ol> <li>Mechanical rooms (large), central plant</li> <li>Roof - Unreduced (see Note 2)</li> <li>Snow Loads</li> </ol>	supporting structure for the loading indicated. Intermediate rails and panel fillers shall be designed to withstand a horizontally applied normal load of 50 pounds on an area not to exceed 12-inches by 12-inches including openings and space between rails and	K. The contractor shall verify depths of piers be delivered to the jobsite in standard lengths a diameter laps in all vertical pier reinforcing, u
to all OSHA guidelines. The Engineer shall not have control of, and shall not be responsible for, construction means, methods, techniques, sequences or procedures, for safety precautions and programs in connection with the Work, for the acts or omissions of the Contractor, Subcontractors, or any other person performing any of	1. Ground snow load, Pg:     5 psf       D. Wind Loads	located so as to produce the maximum load effect. Resulting reactions due to these loads need not be combined with the design loads for handrails or guardrails. See Arch drawings for railing and guardrails.	<ul> <li>Reinforcing steel shop drawings shall include dowels in piers.</li> </ul>
the Work, or for the failure of any of these persons to carry out the Work in accordance with the Structural Contract Documents.	<ol> <li>Wind lateral load on structural frame is based on ASCE7-10 using the following:         <ul> <li>Basic Wind Speed (Ultimate)</li> <li>Exposure</li> <li>Exposure</li> <li>Internal Processor Coefficient COni</li> </ul> </li> </ol>		M. Top of pier shall be of the specified diameter concrete extending beyond the specified dia
H. Where conflict exists among the various parts of the Structural Contract Documents, Structural Drawings, Structural Notes, and Specifications, the strictest requirements, as indicated by the Engineer, shall govern.	<ul> <li>c. Internal Pressure Coefficient, GCpi ± 0.18</li> <li>d. Risk Category III</li> <li>2. Components and cladding wind pressures:2AAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAA</li></ul>	_	N. Temporary steel casing may be required during placement of concrete, any seepage water sl Special construction procedures in accordant accordant and in the followed during outputs.
I. Periodic site observation by field representatives of Encotech is solely for the purpose of determining if the Work is proceeding in accordance with the Structural Contract Documents. This limited site observation is not intended to be a check of the quality	4     3     2     3	ZA	o. All piers shall be inspected by a representative
or quantity of the Work, but rather a periodic check in an effort to inform the Owner against defects and deficiencies in the work of the Contractor.			order to ensure that the proposed bearing m with the recommendations given in the geot
	s contra 2 20NE 2 2 1 2 20NE 4 2	A	<ul> <li>P. The contractor shall make and maintain accubearing stratum, depth of penetration into b (including off center eccentricities), and shal</li> </ul>
	A S A S A S A S A S A S A S A S A S A S		
	NET DESIGN WIND PRESSURE, Pnet, (PSF) FOR COMPONENTS & CLADDING		
	EFFECTIVE WIND AREA (SQ. FT.)		
	ZONE 10 20 50 100 500		
	ZONE 1         16.0 -36.1         16.0 -36.1         16.0 -36.1         16.0 -36.1		
	ZONE 2         16.0 -41.7         16.0 -40.9         16.0 -39.8         16.0 -38.9		
	ZONE 2         16.0         16.0         16.0         16.0           OH         -50.2         -49.3         -48.2         -47.4		
	ZONE 3     16.0     16.0     16.0     16.0       ZONE 3     -55.8     -50.7     -44.0     -38.9       ZONE 3     16.0     16.0     16.0		
	OH     -78.4     -69.9     -58.7     -50.2       30.5     29.1     27.3     26.0     22.8		
	ZONE 4     -33.0     -31.6     -29.9     -28.5     -25.4       ZONE 5     30.5     29.1     27.3     26.0     22.8		
	ZONE 5         -40.6         -37.9         -34.3         -31.6         -25.4		

	ENCOTECH
	ENGINEERING CONSULTANTS
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1141	Austin, Texas 78759   512.338.1101



1	05/18/2021	RFI030
0	01/21/2021	CONFORMED CONTRACT DRAWINGS
ISSUE	DATE	DESCRIPTION



PROJECT MANAGER	W.D. WEHNER
DESIGNED BY	AR / FRV
DRAWN BY	AR
CHECKED BY	FRV
DATE	JANUARY 21, 2021
PROJECT NUMBER	10123906
•	





ical investigation is not possible at this
rations have been completed. The design
nmendations provided by the
nce with nearby sites. A geotechnical
minary recommendations and if
etaining walls will be revised to reflect

7

iteria:

endations in the referenced geotechnical

mestone.

be located on centerline of column nterline of wall or beam.

e using same bar size and number as e beam occurs, use dowels of same size dowels into pier and pilaster or column dowels 30 bar diameters into pier and tructural Drawings.

ise on the Structural Drawings, is at the vall supported by the pier.

rom earth at sides and bottom by sets of the length of the cage and 1'-0" from

mmediately after drilling operations are t cannot be placed by the end of the

ore pier steel is cut. Pier steel may be d cut as required. Provide 64 bar nless noted otherwise in drawings.

placing drawings for templates to set

Form top of pier above ground. Any neter shall be removed.

g pier drilling operations. Prior to the all be removed from the pier holes. e with ACI 336.1 and ACI 336.3R and ion of the casing and during concrete

e of a qualified geotechnical laboratory in terial has been reached in accordance chnical report.

ate records of the drilled pier depths, aring stratum, diameter and location submit this information to the Engineer.

ATTERNS LEGEND	
	SLAB DEPRESSION/ MECH. ZONE
	CMU IN SECTION
	CONCRETE IN SECTION
	CONSTRUCTIC
	EARTH (UNDISTURBE
	EARTH/FILL (COMPACTED)
	GROUT/SAND IN SECTION

8

TRUCTION BY OTHERS

ISTURBED)

H/FILL IPACTED)

JT/SAND CTION

ROCK

STEEL IN SECTION

SHEET LIST SHEET NUMBER SHEET NAME STRUCTURAL NOTES S00-001 S00-002 STRUCTURAL NOTES S00-003 CODE REQUIRED SPECIAL INSPECTIONS AND TESTING S00-004 **3D ISOMETRIC VIEW** S00-005 STRUCTURAL SITE PLAN S30-101 PIER AND TRANSFORMER PAD PLAN S30-102 SWITCHGEAR ENCLOSURE LOADING PLAN S30-103 STRUCTURAL SLAB PLAN S30-104 STRUCTURAL SLAB REINFORCEMENT PLAN ROOF FRAMING PLAN S30-105 S30-106 NET UPLIFT JOIST LOADING PLAN S30-107 SWITCHGEAR ENCLOSURE ELEVATION S30-108 OUTDOOR ELECTRICAL AREA FOUNDATION PLAN S90-201 TYPICAL CONCRETE DETAILS S90-202 TYPICAL CONCRETE DETAILS S90-203 TYPICAL CONCRETE BEAM DETAILS S90-210 CONCRETE FRAMING DETAILS S90-211 CONCRETE FRAMING DETAILS S90-212 FOUNDATION DETAILS S90-301 TYPICAL STEEL DETAILS S90-310 STEEL FRAMING DETAILS S90-401 TYPICAL CMU DETAILS S90-402 TYPICAL CMU DETAILS

## STRUCTURAL NOTES

FILENAME | 18051 Ullrich\_LSPS

SCALE

SHEET S00-001

В

А

D

С

	1					2		3
CAST-IN	I-PLACE CONCRETI	<u>E</u>					<u>Stf</u>	RUCTURAL MASONRY
A. CLA	SSES OF CONCRET	E					A.	Minimum compressive strength of the masonry (f'm) shall be 1,50
	CoA Conc. Class	Min Stren 28-day	gth (psi) 7-day	Agg. Type	Max w/c	Notes	B.	Mortar shall conform to ASTM C270, Type S. Masonry cement sha
	S	4000	2800	NWT	0.45	See note d.	C.	Concrete masonry units shall be hollow load bearing units which a
a.	"NWT" refers to n	ormal conc	rete having	g air dry unit wei	ght of ap	proximately 145		C90, with a minimum net compressive strength as follows.
b.	Maximum water-o	cementitiou	ıs ratio (w/	c) by weight				1 m (psi) Net Area Compressive Strength of CMG
c. d.	"Strength" is requ As described in the	e City of Au	essive cylin Istin Item 4	der strength at a 03S "Concrete fo	on age of or Struct	28 days. ures" Table 5.	_	
	Mix Usage Schedu Desciption of use	lle: Concr	ete Class	CoA Agg. Grade	Slump	Air entrainment	D.	Coarse grout shall conform to ASTM C476, with a maximum aggre a minimum compressive strength equal to the specified minimum strength, f'm, but not less than 2000 psi. Course grout shall be pla with ACI 530.01 3.5.
	Piers, Structural Sl ,Beam, and Slab-o foundation.	ab, S n-grade	4	" (1" nom. size)	1"-3"	4.5%	E.	Reinforce concrete masonry unit joints with ladder type hot dip gasteel conforming to ANSI/ASTM A82, with W1.7 side rods with W1.1. Space joint reinforcing at 16 inches o.c. unless noted otherwis
3. Cen witl	nentitious materia n Items 403S.	ls used in n	nix designs	may be replaced	with fly	ash in accordance	!	<ol> <li>Bar Joint reinforcing 14 metres at spines.</li> <li>Provide prefabricated joint reinforcing corner pieces at all wal intersections.</li> <li>Joint reinforcing shall be discontinuous at control and expansi.</li> </ol>
Hor indi the pro revi add con	izontal construction cated on the Struct center of spans in posed locations fo ew by the Archited itional reinforcing tractor at no addit	on joints in tural Draw accordance r construct ct and Engin as specifie ional cost t	concrete pl ings. All ve e with the t ion joints n neer. Addi d by the En o the owne	acements shall b rtical constructio cypical details. Co ot shown on the tional constructio gineer which sha	be permi on joints ontracto Structur on joints all be pro	tted only where shall be made in or shall submit ral Drawings for may require ovided by the	F.	Lap reinforcing bars in grouted masonry as noted below. Splices i staggered so that not more than 1/2 of all bars are spliced at the s 1. Vertical bars: 60 bar diameters 2. Bond beams: 40 bar diameters 3. Lintels: Do no splice
D. Sub	mittal: Submit mix	designs in	accordanc	e with specificati	ons Item	1 403S	<u>Stf</u>	RUCTURAL STEEL
E. Cor	crete sampling for	quality ass	surance: In	accordance with	n Item 40	035	Α.	Material
ONCR	ETE REINFORCING	. ,						<ol> <li>Ast Not rolled steel members shall be new and conform to Ast</li> <li>ASTM Specification and Grade - clearly mark the grade on each</li> <li>Unloss Noted athenuise on the Structural Drawings, structural</li> </ol>
. Cor 1.	crete reinforceme All reinforcing stee unless noted othe	nt for the p el shall be r rwise in the	project shal new billet s e Structura	l conform to the teel in accordanc I Drawings or the	followin ce ASTM ese notes	g: A615, Grade 60, S.		<ul> <li>a. W-shapes shall conform to ASTM A992.</li> <li>b. Channels shall conform to ASTM A36.</li> <li>c. Angles shall conform to ASTM A36.</li> </ul>
3. Det Det det	ailing of reinforcin ailing Manual and ailing standards, ui	g steel shal all hooks a nless noted	l conform t nd bends ir otherwise	to the American ( reinforcing bars on the Structura	Concrete s shall cc Il Drawir	e Institute 315 onform to ACI ngs.		<ul> <li>a. Square or rectangular hollow structural snape members sr ASTM A500 Grade B, Fy = 46 ksi.</li> <li>e. Structural steel plate shall conform to ASTM A36.</li> <li>f. Any other steel shall conform to ASTM A36.</li> </ul>
2. In u 1. 2. 3. 4. 5.	nscheduled beams Class A lap beam t Class A lap beam t Provide Class B lap Provide standard l walls and slabs. Provide corner ba intersecting beam hooked. Provide 2-#4 diago of steel.	s and slabs, op reinforc oottom reir o at other lo hooks in to rs for all ho s or walls. onal bars at	detail rein ing bars at forcing bars ocation per p bars at ca rizontal ba Corner bar	torcing as follow mid span. rs at the supports ading Engineer's antilever and disc rs at the inside a s are not require entrant corners	s: approva continuo nd outsi d if hori placed u	l. us ends of beams, de faces of zontal bars are nder the top mat	B.	<ol> <li>Fabrication</li> <li>Splicing of structural steel members is prohibited without prior Engineer as to location and type of splice to be made. Any me not shown and detailed on shop drawings will be rejected.</li> <li>Dimensional tolerances of fabricated structural steel shall con of the AISC Code of Standard Practice unless noted otherwise Drawings.</li> <li>Shop painting: Paint structural steel with one coat of manufac oxide primer applied at a rate to provide a uniform dry film th</li> </ol>
D. We Stru	ding of reinforcing Ictural Drawings.	g steel will ı	not be perr	nitted unless spe	cifically	shown on the	C.	<ol> <li>Erection</li> <li>Erection tolerances of anchor bolts, embedded items, and all s unless specified otherwise on the Structural Drawings shall co</li> </ol>
E. Hea	t shall not be used	l in the fabi	rication or i	nstallation of rei	nforcem	ient.		<ol> <li>Code of Standard Practice.</li> <li>Field cutting of structural steel or any field modifications to structural be made without prior approval of the Engineer.</li> </ol>
Rein 1. 2. 3. 4. 5.	nforcing steel clear Beams Columns Drilled Piers One way slabs Slab-on-grade a. "Exterior Expo	r cover shal	l be as follo 1 1/2 1 1/2 3" 3/4" 3/4" s to concre	ows: 2" Int., 2" ext. ex 2" Int., 2" ext. ex top, 3" bottom te exposed to ea	posure posure rth or w	eather		<ol> <li>Contractor shall protect any unprimed structural steel from de corrosion, as required, until the steel is enclosed and protecte construction.</li> <li>Hot dip galvanize after fabrication all structural steel items and permanently exposed to the weather, whether specified on the Drawings or not. Such items include, but are not limited to:         <ul> <li>a. Shelf angles</li> <li>b. All embedded plates in concrete</li> </ul> </li> </ol>
. Sub reir Reii	mittal: Submit sho forcement. Comp nforcement". Do r	p drawings ly with ACI not reprodu	for fabrica 315 "Detai ce the Stru	tion, bending, ar Is and Detailing o ctural Drawings	nd place of Concr for use a	ment of concrete ete as shop drawings.		<ul> <li>c. Enclosure cladding support steel in space not air condition to moisture outside the exterior waterproofing surface if a</li> <li>d. Railing exposed to weather.</li> <li>e. Examine the Architectural and Structural Drawings for oth be hot dipped galvanized. Galvanize all nuts, bolts, and wa connection with such steel. Field welded connections shal protected with "Z.R.C. Cold Galvanizing Compound" as ma Company.</li> </ul>
							D.	Contractor shall coordinate structural steel fireproofing requirem structural steel, including steel joists, scheduled or indicated to re- fireproofing shall be delivered to the project site unprimed. Steel conditions after installation shall be primed with a protective coat diminish the bond between the spray applied fireproofing, and the Any primer, and/or coating applied to structural steel shall be app applicable U.L. Fire Resistance Assembly used on the project.
							E.	Submittal: Provide drawings showing details for fabrication and sh

ation and she members, erection plans and details. Include details of connections, camber, weld profiles and sizes and spacing. Shop and erection drawings shall not be made using reproductions of the Structural Drawings.

		PROJECT MANAGER	W.D. WEHNER
		DESIGNED BY	AR / FRV
		DRAWN BY	AR
		CHECKED BY	FRV
0 01/21/2021	CONFORMED CONTRACT DRAWINGS	DATE	OCTOBER 2020
ISSUE DATE	DESCRIPTION	PROJECT NUMBER	10123906

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 Austin, Texas 78759 | 512.338.1101



Texas P.E. Firm Registration No. F-754

		4	5		6			7	8	-
		STRUCTURAL STEEL CONNECTIONS		METAL DECKS						
nall be 1,500 pounds per s	square	<ul> <li>A. Welded Connections</li> <li>1. All welding shall conform to ANSI/AWS E</li> <li>2. Fillet welds with no size specified shall be</li> </ul>	01.1, latest edition e 3/16 inch or minimum size required by	A. Metal Roof Deck 1. Metal Roof Deck Scheo Location Gauge	dule: SDI Deck Deck Sheet	Min. Mir	n. Min.			
cement shall not be used.		AISC, whichever is larger.			Type Depth Width (in) (in)	lx Sp (in4) (in3	Sn 3) (in3)			
iits which conform to ASTI ws: gth of CMU Block (psi)	M	<ul> <li>B. Bolted Connections</li> <li>1. Unless noted otherwise on the Structura diameter and conform to ASTM A325. B bearing type bolts with thread allowed in</li> <li>2. Bolts shall be tightened to "snug tight" a on the Structural Drawings.</li> </ul>	l Drawings, bolts shall be 3/4 inch olts shall be designed using values for n the shear plane. s defined by AISC, unless noted otherwise	Typ. UNO 22 Sp = positive section m Sn = negative section r I = moment of inertia i	WR 1.5 36 nodulus in3 nodulus in3 n4	0.169 0.18	86 0.194			D
num aggregate size of 1/2 I minimum compressive shall be placed in accorda	2" and ince	C. Structural steel connections not specifically of be designed and detailed by the Contractor of professional engineer licensed in the state has Sealed calculations for all connections design for the Architect's files.	detailed on the Structural Drawings shall under the direct supervision of a aving jurisdiction at the project site. ned by the Contractor shall be submitted	<ol> <li>Roof deck shall be galvania</li> <li>Sheet steel for galvania</li> <li>Structural Quality, with conform to ASTM A653</li> <li>Sheet steel for prime p A1008 with a minimum</li> </ol>	vanized. zed roof deck and accessories sh n a minimum yield strength of 33 3 with a minimum coating of G6 painted roof deck and accessorie n yield strength of 33 ksi.	all conform to a 3 ksi. Galvanizin 0 as defined in s shall conform	ASTM A653, ng shall A653. n to ASTM			
e hot dip galvanized cold-c ds with W1.7 cross rods. d otherwise.	drawn	<ul> <li>Roof edge angles shall be continuous and sh shall be butt welded to develop full capacity</li> </ul>	all be spliced only at supports. Splices of the member.	<ol> <li>Roof deck shall be con</li> <li>Place deck panels on s lapped 2 inches over s</li> </ol>	tinuous over four or more suppo tructural supports and adjust to tructural supports. Provide mini	orts. final position w imum end bear	vith ends ing of 2			
s at all wall corners and		E. For connections not specifically addressed by provide fillet welds at all contact surfaces su	y these notes or the Structural Drawings, fficient to develop the tensile strength of	7. Roof deck connections	shall be as follows:	lan Eastonor/				
v. Splices in reinforcing sh	hall be			Location Pat	tern Fastener Nor	per span				
red at the same location.		A. Open web steel joists shall conform to the St	andard Specifications of the Steel Joist	Interior Field Perimeter Band	36/4 5/8 PW #10 36/7 5/8 PW #10	Tek/ 3 Tek/ 5				
		<ul> <li>B. Provide bridging in accordance with SJI Spec CFR-1926.757(C). Bridging shall be continuo shall be anchored to spandrel members or w required for uplift.</li> </ul>	ifications and OSHA Standard 29 us through structural steel members, and valls. Provide additional bridging where	Ridge Band Corner Zones See Design Wind Load Perimeter Band, Ridge PW = Arc Spot Weld (P	36/7 5/8 PW #10 36/7 5/8 PW #10 information or plans for "a" dim Band, and Corner Zones wind lo	Tek/ 5 Tek/ 5 nension and Inte pads.	erior Fields,			С
orm to ASTM specification ade on each member.	ı A6. s shall	C. See Joist Loading Plan for joist design wind p	ressures.	<ul><li>a. 0.1 times the enclose</li><li>b. 0.4 times the eave</li><li>c. Minimum width of</li></ul>	osure lesser plan dimension. height. 4 feet.					
, structural steel members	3 311011	D. Joist manufacturer shall design chords of jois applied load of 100 pounds between all pane reinforcing. This additional load has been ac and is not additive to those specified.	sts to support a nominal concentrically- el points without requiring additional scounted for in the overall design loads	8. Power driven fastener deck gauge and deck s complete manufacture	s shall be selected by the Contra upport member thickness. Subr er's information, including diaph	ctor for the cor nit proposed fa ragm shear valu	mbinations of asteners with ues for the			
nembers shall conform to 6.		E. All hangers or attachments to joists shall be bottom chord(s). Hangers with reactions in the panel points of the joist, or the chord(s) "Typical Details."	placed concentric with the top and excess of 100 pounds must be located at shall be reinforced in accordance with the	9. Arc Spot Weld shall be washers for decking lig 10. Mechanical, electrical roof deck.	5/8" minimum diameter and sh ghter than 22 gauge. and plumbing systems shall not	all be made thr be supported b	rough weld by the metal fastening			
ithout prior approval of th le. Any member having sp jected.	he plice	F. Provide flat bearing for all joists. Bear joists Specifications.	on supports in accordance with SJI	methods and layout, s and any other pertiner	upport locations, projections, op nt details and accessories.	penings and reir	nforcement,			
el shall conform to Section otherwise on the Structur	n 6.4 ral	<ul> <li>G. Joists shall be connected to their supports in indicated by the joist manufacturer.</li> </ul>	accordance with SJI Specifications and as	DESIGN BY OTHERS	acifications the items listed hele		dad in the			
of manufacturer's standard dry film thickness of 2.5 m	d red nils.	<ul> <li>H. Shop painting: Joists shall receive a shop-coa inhibitive primer conforming to the Steel Str SSPC No. 15, or better.</li> </ul>	at of the joist manufacturer's standard rust uctures Painting Council Specification,	A. In accordance with the spectrum Contract Documents. Des Contractor, and shall be de licensed in the state having 1. Embedded assemblies	ign of these elements shall be the esigned and sealed by a registere g jurisdiction at the project site. and inserts, clamps, hangers, tra	e responsibility ed professional apezes, unistrut	y of the engineer t, etc. for the			В
ns, and all structural steel gs shall conform to the AI	ISC	<ol> <li>Submittals: Submit shop drawings including standard designations, bridging, camber, ma extensions and connection details. Submit c</li> </ol>	setting plans and shop tickets indicating terial strengths, spacing, joist seat alculations for all joists for which standard	support of MEP system 2. Embedded assemblies	ns. , inserts, and/or hangers for fire	suppression sy	vstems.			
itions to structural steel sh r.	hall	drawings. Calculations shall be signed and so	ealed by a registered professional	B. Design of the items listed a Code, and shall include all	above shall be in accordance wit attachments to the structure.	h the General E	Building			
d protected by the new	CTS OT	be retained for the Architects file and shall n	ot be approved or returned.							
el items and connections cified on the Structural nited to:										
r conditioned and/or expc surface if any.	osed									
ngs for other items require olts, and washers used in ctions shall have welds und" as manufactured by 2	ed to Z.R.C.									
requirements. All interior cated to receive spray app ned. Steel exposed to corr ective coating which does ing, and the steel substrate hall be approved for use in roject.	ior blied crosive s not te. n the									A
tion and shop assembly of	f									





ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL



The following Statement and Schedules of Inspectio shall be performed for this project. Special Inspector	ns are those Special Inspections and Tests that rs shall reference these plans and IBC Chapter 17	SCHEDULES OF SPECIAL INSPECTIONS	:
or all special inspection requirements.		REQUIRED SPECIAL INSPECT	IONS AND TES
he owner shall retain an "approved agency" per الم	C 1703 to provide special inspections for this	VERIFICATION AND INSPECTION T	ASK F
project. Special Inspectors shall be qualified persons	per IBC 1704.2.1. Special inspection reports shall		CO
he Authority Having Jurisdiction for review. In addit reports and certificates noted in IBC 1704.5 to the A nspection reports will be required by each special ir	tion to special inspection reports and tests, submit uthority Having Jurisdiction. Final special hspection firm per IBC 1704.2.4.	<ol> <li>Installation of open-web steel joists a girders.</li> <li>a. End Connections — welding or bolted</li> <li>b. Bridging — horizontal or diagonal</li> </ol>	nd joist
TATEMENT OF SPECIAL INSPECTIONS: This statement of Special Inspections has been writt Official will:	en with the understanding that the Building	2. Standard bridging	
<ul> <li>Review and approve the qualifications of the Special inspection activity on the program qualified and performing their duty as state with</li> </ul>	ecial Inspectors oject site to assure that Special Inspectors are nin this statement.	<ol> <li>Bridging that differs from the SJI spec listed in Section 2207.1</li> </ol>	ifications
inspections as required by IBC Section 110.3.	to them by the Special Inspector Perform		
		REQUIRED SPECIAL INSPEC	FREQ
Special inspection of shop fabricated, structural load	I bearing members shall be verified by the Special	VERIFICATION AND INSPECTION TASK	CONTINUOUS
nspector per Section 1704.2.5		<ol> <li>Inspect reinforcement, including pre-stressing tendons, and verify</li> </ol>	_
Special inspection of Structural Steel shall be per IBC	Section 1705.2.1. A qualified Special Inspector of	Princement.	
an "approved agency" providing Quality Assurance ( and confirm the Fabricator and Erector's Quality Con	QA) Special Inspections for the project shall review htrol (QC) procedures for completeness and	<ul> <li>a. Verify weldability of reinforcing bars other than ASTM A706.</li> </ul>	_
adequacy relative to AISC 360-10 Chapter N, AISC 30 Structural Welding Code, and 2015 IBC code require	ments for the fabricator's scope of work.	b. Inspect single pass fillet weld	_
QA Agency providing Special Inspections sha	Il provide personnel meeting the minimum	maximum 5/16".	
qualification requirements for Inspection and Section N4.	a Nondestructive Testing NDT per AISC 360-10	<ul> <li>c. Inspect all other welds.</li> <li>3 Inspect anchors cast in concrete</li> </ul>	X
<ul> <li>Verify Fabricator and Erector QC Program period</li> </ul>	er AISC 360-10 Section N2.	4. Inspect anchors post-installed in	
<ul> <li>Inspection of welds and bolts by both QC and Special Inspections below. All provisions of A statically leaded structures shall evenly</li> </ul>	d QA personnel shall be per the Schedule of WS D1.1-2010 Structural Welding Code for	hardened concrete members: a. Adhesive anchors installed in	
<ul> <li>statically loaded structures shall apply.</li> <li>Nondestructive Testing (NDT) of welds:</li> </ul>		horizontally or upwardly inclined	x
Non-Destructive Testing (NDT) of welder	d joints per AISC 360-10 N5.5.	tension loads.	
<ul> <li>Risk Category for determination of exten Loads section of the Structural Notes.</li> <li>NDT performed shall be documented an</li> </ul>	It of NDT per AISC 360 N5.5b is noted in the Design	b. Mechanical anchors and adhesive anchors not defined in 4a.	_
mark and location of the piece.	a reports shall dentify the tested weld by piece	5. Verify use of required design mix.	
For field work, the NDT report shall iden	tify the tested weld by location in the structure,		
<ul> <li>piece mark and location of the piece.</li> <li>Additional Inspection tasks per AISC 360-10</li> </ul>	Section N5.7	<ul> <li>6. Prior to concrete placement, fabricate specimens, for strength tests, perform slump and air content</li> </ul>	×
Special INSPECTION OF COLD-FORMED STEEL DECH Special inspections and qualification of welding spec	<u>K</u> ial inspectors for cold-formed steel roof deck shall	tests, and determine the temperature of the concrete.	
e in accordance with the quality assurance and insp PECIAL INSPECTION OF CONCRETE CONSTRUCTION	bection requirements of SDI QA/QC.	7. Inspect concrete and shotcrete placement for proper application	x
pecial inspection and tests of concrete construction	shall be performed in accordance with this	techniques.	
<ul> <li>Special inspections shall not be required for:</li> <li>Isolated spread concrete featings of one</li> </ul>	ons:	8. Verify maintenance of specified curing temperature and techniques.	
fully supported on earth or rock.	iosures three stories or less above the grade plane	a. Application of prestressing forces	x
2. Continuous footings supporting walls of	enclosures three stories or less above the grade	b. Grouting of bonded prestressing	Y
plane that are fully supported on earth o a. The footings support walls of light fr	ame construction.	tendons.	^
<ul><li>b. The footings are designed in accorda</li><li>c. The structural design of the footing i</li></ul>	nce with IBC Table 1809.7. s based on a specified compressive strength, f' <sub>c</sub> ,	members.	
not more than 2,500 psi.	ly on the ground including prostranged date are	11. verity in-situ concrete strength, prior to stressing of tendons in	
grade, where the effective prestress in t	ne concrete is less than 150 psi	post-tensioned concrete and prior to	-
4. Concrete foundation walls constructed in	n accordance with Table 1807.1.6.2.	removal of shores and forms from beams and structural slabs.	
5. Concrete patios, driveways, and sidewall	ks, on grade.	12. Inspect formwork for shape, location	
<b><u>POST-INSTALLED ANCHORS TO CONCRETE AND MA</u></b> Special Inspections of post-installed anchors to conc	SONRY: rete and masonry shall comply with IBC Section	and dimensions of the concrete member being formed.	_
L/U3. Inspections shall be in accordance with the red Evaluation Report and as indicated by the design red	quirements set forth in the approved ICC guirements specified on the drawings. Refer to the		
POST-INSTALLED ANCHORS AND DOWELS section of	the Structural Notes for anchors that are the basis		
of the design. Special inspector shall verify anchors a AND DOWELS section of the Structural Notes or as a require approval by the SER and require substantiat	are as specified in the POST-INSTALLED ANCHORS therwise specified on the drawings. Substitutions	VERIFICATION AND INSPECTIO	IAL INSPECTIC
CC Evaluation Services (ES) Report. Special Inspecto	r shall document in their Special Inspection Report	1. Verify materials below shallow found	ations are
compliance with each of the elements required with Report	in the applicable ICC Evaluation Services (ES)	adequate to achieve the design bearing	ng capacity.
PREFABRICATED CONSTRUCTION		<ol> <li>Verify excavations are extended to pr have reached proper material</li> </ol>	oper depth and
Special Inspections of all prefabricated construction	shall conform to IBC Section 1703.	3. Perform classification and testing of c	ompacted fill
		4. Verify use of proper materials, densiti	ies and lift
		thickness during placement and comp	paction of
		thickness during placement and comp compacted fill	inspect subgrad

Œ	ENCOTECH
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Texas P.E. Firm	0
Registration No. F-754	ISSUE

01/21/2021 DATE

CONFORMED CONTRACT DRAWINGS DESCRIPTION

	TABLE 1705.2.3	}		
) .	TESTS OF STEEL			
	FREQUENCY DURING TASK		REFERENCED	REQUIRED?
	CONTINUOUS	PERIODIC	STANDARD	1718
	_	х —	SJI specifications listed in Section 2207.1	Y
	_	х	SJI specifications listed in Section 2207.1	Y
		х		Y

TA	BLE 1705.3			1
ND TES	STS OF CONCRE	TE CONSTRUCTION		
FREQUENCY		REFERENCED	IBC	Y/N
JOUS PERIODIC		STANDARD	REFERENCE	-
		AC I 318 Ch. 20,		
	Х	25.2, 25.3,	1908.4	Y
		26.6.1-26.6.3		
	х			
		AWS D1.4	_	N
	х	ACI 318: 26.6.5		
	_			
	X	ACI 318: 17.8.2	_	Y
	_	AC I 318: 17.8.2.4	_	Y
	Х	AC   318: 17.8.2	_	
	х	ACT 318: Ch. 19,	1904.1-3	Y
		20.4.5, 20.4.4		
		ASTM C172	1008 10	v
	_	ACI 318. 26.12	1908.10	T
		/(01010.20.12		
	_	ACI 318: 26.5	1908.6-8	Y
				-
		ACI 318 :26.5.3 -		
	Х	26.5.5	1908.9	Y
	_		_	N
		ACI 318: 26.10		
	_		—	N
	V	ACI 210: 20 11 2		NI
	X	ACI 318: 26.11.2	—	N
	Х	ACI 318: 26.11.2	_	Y
		ACI 318:		
	Х	26.11.1.2(b)	—	Y
				1

LE 1705.6				
	AND TESTS OF SOILS			
	FREQUENCY DUR	RING TASK LISTED		
	CONTINUOUS	PERIODIC		
e ty.	_	х	Y	
th and	_	х	Y	
d fill	_	х	Y	
t	Х	_	Y	
ıbgrade /.	_	Х	Y	

			FREQ		NG TASK LISTED	REQUIRED?
			CONTIN	NUOUS	PERIODIC	
1.	Inspect drilling operation and maintai accurate records for each element.	n complete and	×	<	_	Υ
2.	Verify placement locations and plumb element diameters, bell diameters (if adequate end-bearing strata capacity or grout volumes.	oness, confirm applicable), and . Record concret	e X	<	_	Y
3.	For concrete elements, perform tests special inspections in accordance with	and additional	_	_	_	Y
	REQUIRED SPECIAL INSI	TAE PECTIONS AND 1	BLE 1705.4 FESTS OF MAS	ONRY CONST	RUCTION - LEVEL B	
		MINI	MUM TESTS			
1.	Verification of Slump flow and Visual 5.B.1.b.3 for self-consolidating grout V Exc	Stability Index (\ erification of f'm ept where speci	/SI) as delivered and f'aac in ac fically exempt l	d to the proje ccordance wi by the Code	ect site in accordance th Article 1.4B prior to	with Article construction,
V	ERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC	TMS40 ACI 530 ACI 53	2/ TMS602/ D/ ACI 530.1/ 5 ASCE 6	REQUIRED? Y/N
1.	Verify compliance with the approved submittals	_	Х		Art. 1.5	Y
2.	As masonry construction begins, verify that the following are in compliance:					
a.	Proportions of site-prepared mortar.	_	Х		Art. 2.1, 2.6 A	Y
b.	Construction of mortar joints.	_	х		Art. 3.3 B	Y
c.	Grade and size of pre-stressing tendons and anchorages.	_	Х		Art. 2.4 B, 2.4 H	N
d.	Location of reinforcement, connectors, and pre-stressing tendons and anchorages	_	Х		Art. 3.4, 3.6 A	Y
e. f	Pre-stressing technique	_	Х		Art. 3.6 B	N
3.	AAC masonry Prior to grouting, verify that the	X(a)	X(b)		Art. 2.1 C	N
	following are in compliance:					
a.	Grout space.	_	х		Art. 3.2 D, 3.2 F	Y
b.	Grade, type and size of reinforcement and anchor bolts, and pre-stressing tendons and anchorages.	_	х	Sec. 6.	1 Art. 2.4, 3.4	Y
c.	Placement of reinforcement, connectors, and pre-stressing tendons and anchorages	_	Х	Sec. 6.1, 6 6.2.6, 6.	5.2.1, Art. 3.2 E, 3.4, 2.7 3.6 A	Y
d.	Proportions of site-prepared grout and prestressing grout for bonded tendons.	_	х		Art. 2.6B, 2.4G.1.b	Y
е. 4.	Construction of mortar joints. Verify during construction:	_	Х		Art. 3.3 B	Y
a.	Size and location of structural elements.	_	х		Art. 3.3 F	Y
b.	Type, size, and location of anchors, including other details of anchorage of masonry to structural members, frames, or other construction.	_	Х	Sec. 1.2.1 6.1.4.3, 6	L(e), 5.2.1	Y
c.	Welding of reinforcement.	х	_	Sec. 8.1.6 9.3.3.4( 11.3.3.4	.7.2, c), (b)	N
u.	protection of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F).	_	Х		Art. 1.8 C, 1.8 D	Y
e.	Application and measurement of prestressing force.	x	_		Art. 3.6 B	N
f.	Placement of grout and prestressing grout for bonded tendons is in compliance.	x	_		Art. 3.5, 3.6 C	N
g.	Placement of AAC masonry units and construction of thin-bed mortar joints	X(a)	X(b)		Art. 3.3 B.9, 3.3 F.1.b	N
5.	Observe preparation of grout specimens, mortar specimens, and/or prisms	_	Х		Art. 1.5	N

TABLE 1705.8

REQUIRED SPECIAL INSPECTIONS AND TESTS OF CAST-IN-PLACE DEEP FOUNDATION ELEMENTS

(b) Required after the first 5000 square feet of AAC masonry

PROJECT MANAGER	W.D. WEHNER
DESIGNED BY	AR / FRV
DRAWN BY	AR
CHECKED BY	FRV
DATE	OCTOBER 2020
PROJECT NUMBER	10123906





**ULLRICH WTP** LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL

	1		8	
	REQUIRED SPECIAL INSPECTIONS FOR STEEL	CONSTRUCTION		
	AISC 360 TABLE N5.4-1: INSPECTION TASKS PRIOR TO WELDING	QC	QA	REQUIRED?
	Welding procedure specifications (WPSs) available	P	Р	Y
<u>2.</u>	Manufacturer certifications for welding consumables available	P	P	Y
8.	Material identification (type/grade)	0	0	Y
ŀ	Welder identification system	0	0	Y
<b>.</b>	Fit-up of groove welds (including joint geometry)			
а.	Joint preparation			
b.	Dimensions (alignment, root opening, root face, bevel)	0	0	Y
с. а	Cleanliness (condition of steel surfaces)			
u.	Backing type and fit (if applicable)			
с. ;	Configuration and finish of access holes	0	0	Y
'.	Fit-up of fillet welds			
a.	Dimensions (alignment, gaps at root)			
b.	Cleanliness (condition of steel surfaces)	0	0	Y
c.	Tacking (tack weld quality and location)			
	Check welding equipment	0	_	Y
	AISC 360 TABLE N5.4-2:	ос	QA	
	INSPECTION TASKS DURING WELDING		~.	
	Use of qualified welders	0	0	Y
	Control and handling of weiding consumables	0	0	V
a. h	Fackaging Exposure control	0	0	I
	No welding over cracked tack welds	0	0	Y
<u>.</u> ۱.	Environmental conditions			
a.	Wind speed within limits	0	0	Y
b.	Precipitation and temperature			
	WPS followed			
a.	Settings on welding equipment			
b.	Travel speed			
c.	Selected welding materials	0	0	Y
d.	Shielding gas type/flow rate		_	
e.	Preheat applied			
t.	Interpass temperature maintained (min./max.)			
g.	Wolding techniques			
י. א	Internass and final cleaning			
b.	Fach pass within profile limitations	0	0	Y
c.	Each pass meets guality requirements			
-	AISC 360 TABLE N5.4-3:			
	INSPECTION TASKS AFTER WELDING	QC	QA	
	Welds cleaned	0	0	Y
2.	Size, length and location of welds	Р	Р	Y
	Welds meet visual acceptance criteria			
a. h	Wold /base motal fusion			
ט. ר	Crater cross section			
d.	Weld profiles	Р	Р	Y
e.	Weld size			
f.	Undercut			
g.	Porosity			
ŀ.	Arc strikes	Р	Р	Y
	k-area	Р	Р	Y
	Backing removed and weld tabs removed (if required)	P -	P -	Υ
).		P	P	Y
). /.	Repair activities	п	п	v
) / }	Document acceptance or rejection of welded joint or member AISC 360 TABLE N5.6-1	Р	Р	Y
) / 5	AISC 360 TABLE N5.6-1: INSPECTION TASKS PRIOR TO BOLTING	Р <b>QC</b>	P QA	Y
· ·	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials	Р <b>QC</b> О	Р <b>QA</b> Р	Y
<u>.</u>	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements	Р <b>QC</b> О О	Р <b>QA</b> Р О	Y N N
<u>.</u>	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if	Р QC О О	Р <b>QA</b> Р О	Y N N
······································	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)	Р QC О О О	Р <b>QA</b> Р О О	Y N N N
<u>.</u>	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper bolting procedure selected for joint detail	Р QC О О О О	Р <b>QA</b> Р О О О	Y N N N N N N N
······································	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper fasteners selected for the joint detail         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)	Р QC О О О О О	Р <b>QA</b> Р О О О О	Y  N N N N N N N N N
2. 2. 3.	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper fasteners selected for the joint detail         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Pre-installation verification testing by installation personnel observed and	Р QC О О О О О О	Р QA Р О О О О	Y  N N N N N N N N
······································	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper fasteners selected for the joint detail         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used	Р QC О О О О О О О О О О Р	Р QA Р О О О О О	Y  N N N N N N N N N N N N N N
	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper fasteners selected for the joint detail         Proper fasteners selected for ble second from shear plane         Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used         Proper storage provided for bolts, nuts, washers and other fastener	Р QC О О О О О О Ф Р	Р <b>QA</b> Р О О О О О О	Y N N N N N N N N N N
	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used         Proper storage provided for bolts, nuts, washers and other fastener components	Р QC О О О О О О О О О О О О О	Р QA Р О О О О О О	Y  N N N N N N N N N N N N N N N N N N
j.	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used         Proper storage provided for bolts, nuts, washers and other fastener components         AISC 360 TABLE N5.6-2:	Р QC 0 0 0 0 0 0 0 0 0 0 0 0 0	Р QA Р О О О О О О О О О	Y  N N N N N N N N N N N N N
5. 7. 5. 5.	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper bolting procedure selected for joint detail         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used         Proper storage provided for bolts, nuts, washers and other fastener components         AISC 360 TABLE N5.6-2:         INSPECTION TASKS DURING BOLTING	Р QC О О О О О О О О О О О О О	Р QA Р О О О О О О О О О О О О О	Y  N N N N N N N N N N N N N N N N N N
5. 7. 3. 5. 5. 7.	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper bolting procedure selected for joint detail         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used         Proper storage provided for bolts, nuts, washers and other fastener components         AISC 360 TABLE N5.6-2:         INSPECTION TASKS DURING BOLTING         Fastener assemblies, of suitable condition, placed in all holes and washer (if required) are positioned as required	Р QC О О О О О О О О О О О О О	Р QA Р О О О О О О О О О О О О О О	Y  N N N N N N N N N N N N N N N N N N
7.       7.       3.       L.       2.       3.       4.       5.       7.	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper bolting procedure selected for joint detail         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used         Proper storage provided for bolts, nuts, washers and other fastener components         AISC 360 TABLE N5.6-2:         INSPECTION TASKS DURING BOLTING         Fastener assemblies, of suitable condition, placed in all holes and washer (if required) are positioned as required	Р QC 0 0 0 0 0 0 0 0 0 0 0 0 0	Р QA Р О О О О О О О О О О О О О	Y  N N N N N N N N N N N N N N N N N N
	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper bolting procedure selected for joint detail         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used         Proper storage provided for bolts, nuts, washers and other fastener components         AISC 360 TABLE N5.6-2:         INSPECTION TASKS DURING BOLTING         Fastener assemblies, of suitable condition, placed in all holes and washer (if required) are positioned as required         Joint brought to the snug-tight condition prior to the pretensioning operation	Р QC О О О О О О О О О О О О О	Р QA Р О О О О О О О О О О О О О	Y  N N N N N N N N N N N N N N N N N N
7.       3.       1.       2.       3.       5.       7.       1.       2.       3.	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper bolting procedure selected for joint detail         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used         Proper storage provided for bolts, nuts, washers and other fastener components         AISC 360 TABLE N5.6-2:         INSPECTION TASKS DURING BOLTING         Fastener assemblies, of suitable condition, placed in all holes and washer (if required) are positioned as required         Joint brought to the snug-tight condition prior to the pretensioning operation         Fastener component not turned by the wrench prevented from rotating	Р QC 0 0 0 0 0 0 0 0 0 0 0 0 0	Р QA Р О О О О О О О О О О О О О	Y Y N N N N N N N N N N N N N N N N N N
J.       7.       3.       1.       2.       3.       5.       5.       5.       7.       1.       2.       3.       4.       2.       3.       4.       3.       4.       3.       4.	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper fasteners selected for the joint detail         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used         Proper storage provided for bolts, nuts, washers and other fastener components         AISC 360 TABLE N5.6-2:         INSPECTION TASKS DURING BOLTING         Fastener assemblies, of suitable condition, placed in all holes and washer (if required) are positioned as required         Joint brought to the snug-tight condition prior to the pretensioning operation         Fastener component not turned by the wrench prevented from rotating         Fasteners are pretensioned in accordance with the RCSC Specification,	Р QC 0 0 0 0 0 0 0 0 0 0 0 0 0	Р QA Р О О О О О О О О О О О О О	Y Y N N N N N N N N N N N N N N N N N N
3.       7.       3.       3.       2.       3.       5.       5.       7.       1.       2.       3.       1.       2.       3.       1.       3.       1.       3.       1.       3.       1.       3.       1.	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper fasteners selected for the joint detail         Proper fasteners selected for bolts, nuts, washers plane)         Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used         Proper storage provided for bolts, nuts, washers and other fastener components         AISC 360 TABLE N5.6-2:         INSPECTION TASKS DURING BOLTING         Fastener assemblies, of suitable condition, placed in all holes and washer (if required) are positioned as required         Joint brought to the snug-tight condition prior to the pretensioning operation         Fastener component not turned by the wrench prevented from rotating         Fasteners are pretensioned in accordance with the RCSC Specification, progressing systematically from the most rigid point toward the free edget	Р QC 0 0 0 0 0 0 0 0 0 0 0 0 0	Р QA Р О О О О О О О О О О О О О	Υ Ν Ν Ν Ν Ν Ν Ν Ν Ν Ν Ν Ν Ν
J.         7.         3.         3.         2.         3.         5.         7.         5.         7.         1.         2.         3.         4.         5.         7.         1.         2.         3.         4.         3.         4.	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used         Proper storage provided for bolts, nuts, washers and other fastener components         AISC 360 TABLE N5.6-2:         INSPECTION TASKS DURING BOLTING         Fastener assemblies, of suitable condition, placed in all holes and washer (if required) are positioned as required         Joint brought to the snug-tight condition prior to the pretensioning operation         Fastener component not turned by the wrench prevented from rotating         Fasteners are pretensioned in accordance with the RCSC Specification, progressing systematically from the most rigid point toward the free edge	Р QC 0 0 0 0 0 0 0 0 0 0 0 0 0	Р QA Р О О О О О О О О О О О О О	Y  N N N N N N N N N N N N N N N N N N
3.         7.         3.         1.         2.         3.         5.         5.         7.         1.         2.         3.         4.         3.         4.         3.         4.         3.         4.         3.         4.	Repair activities         Document acceptance or rejection of welded joint or member         AISC 360 TABLE N5.6-1:         INSPECTION TASKS PRIOR TO BOLTING         Manufacturer's certifications available for fastener materials         Fasteners marked in accordance with ASTM requirements         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Proper bolting procedure selected for joint detail         Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)         Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used         Proper storage provided for bolts, nuts, washers and other fastener components         AISC 360 TABLE N5.6-2:         INSPECTION TASKS DURING BOLTING         Fastener assemblies, of suitable condition, placed in all holes and washer (if required) are positioned as required         Joint brought to the snug-tight condition prior to the pretensioning operation         Fastener component not turned by the wrench prevented from rotating         Fasteners are pretensioned in accordance with the RCSC Specification, progressing systematically from the most rigid point toward the free edge	Р QC 0 0 0 0 0 0 0 0 0 0 0 0 0	Р QA Р О О О О О О О О О О О О О	Υ Ν Ν Ν Ν Ν Ν Ν Ν Ν Ν Ν Ν Ν

## CODE REQUIRED SPECIAL INSPECTIONS AND TESTING

0 1"

FILENAME 18051 Ullrich\_LSPS

SCALE

SHEET 55 of 350 S00-003



	ENCOTECH
	ENGINEERING CONSULTANTS
TBPE Firm 1141	8500 Bluffstone Cove, Suite B-103 Austin, Texas 78759   512.338.1101



0	01/21/2021	CONFORMED CONTRACT DRAWINGS
ISSUE	DATE	DESCRIPTION

PROJECT MANAGER	W.D. WEHNER
DESIGNED BY	AR / FRV
DRAWN BY	AR
CHECKED BY	FRV
DATE	OCTOBER 2020
PROJECT NUMBER	10123906





ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL

0 1"

FILENAME SCALE

2"

SHEET 56 of 350



	ENGINEERING CONSULTANTS
TBPE Firm 1141	8500 Bluffstone Cove, Suite B-103 Austin, Texas 78759   512.338.1101



0	01/21/2021	CONFORMED CONTRACT DRAWINGS
ISSUE	DATE	DESCRIPTION

PROJECT NUMBER 10123906



Texas P.E. Firm Registration No. F-754	

0 ISSUE	01/21/2021 DATE	CONFORMED CONTRACT DRAWINGS DESCRIPTION	

PROJECT MANAGER	W. D. WEHNER
DESIGNED BY	LEM
DRAWN BY	HDG
CHECKED BY	CAZ
DATE	OCTOBER 2020
PROJECT NUMBER	10123906





ULLRICH WTP





GENERAL NOTES:

- 1. SEE STD DETAILS AND SCHEDULES FOR RETAINING WALL DETAILS AND REINFORCING.
- 2. SPACE PARTIAL CONTROL JOINTS (PCJ) AT 30'-0" O.C. AND EXPANSION JOINTS (EJ) AT 90'-0" O.C. UNO.
- 3. SEE CIVIL FOR NORTHING AND EASTINGS OF ALL CANTILEVER RETAINING WALLS.
- 4. CONTRACTOR SHALL SUBMIT WALL JOINTING PROPOSAL FOR ALL WALLS PRIOR TO CONSTRUCTION.







	-22
Te>	kas P.E. Firm
Reថ	gistration No. F-754

ISSUE	DATE	DESCRIPTION
0	01/21/2021	CONFORMED CONTRACT DRAWINGS

PROJECT MANAGER	W. D. WEHNER
DESIGNED BY	LEM
DRAWN BY	HDG
CHECKED BY	CAZ
DATE	OCTOBER 2020
PROJECT NUMBER	10123906





ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL



GENERAL NOTES:

1. SEE STD DETAILS AND SCHEDULES FOR RETAINING WALL DETAILS AND REINFORCING.

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С

- 2. SPACE PARTIAL CONTROL JOINTS (PCJ) AT 30'-0" O.C. AND EXPANSION JOINTS (EJ) AT 90'-0" O.C. UNO.
- 3. SEE CIVIL FOR NORTHINGS AND EASTINGS OF ALL CANTILEVER RETAINING WALLS.
- 4. CONTRACTOR SHALL SUBMIT WALL JOINTING PROPOSAL FOR ALL WALLS PRIOR TO CONSTRUCTION.

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Texas P.E. Firm Registration No. F-754	

0	01/21/2021	CONFORMED CONTRACT DRAWINGS
SSUE	DATE	DESCRIPTION







ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL

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## GENERAL NOTES:

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- 1. REFERENCE CIVIL FOR PROPOSED PIPING PENETRATIONS AND SEDIMENTATION/ FILTRATION POND PROFILE.
- 2. REFERENCE CIVIL SITE PLAN FOR STRUCTURE LOCATION.









FJS
Texas P.E. Firm Registration No. F-754

SUE	DATE	DESCRIPTION
0	01/21/2021	CONFORMED CONTRACT DRAWINGS



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LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL



FILENAME S10-201.dwg **SCALE** 1" = 10'

**SHEET** 64 of 350 S10-203

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DATE OCTOBER 2020

PROJECT NUMBER 10123906

01/21/2021



0	01/21/2021	CONFORMED CONTRACT DRAWINGS
ISSUE	DATE	DESCRIPTION



- 1. REFERENCE CIVIL FOR SITE CONTROL POINTS AND WALL RADII, TYP ALL RETAINING WALLS.
- 2. REFERENCE CIVIL FOR SITE GRADING, LAYDOWN CURB, AND CURB AND GUTTER DETAILS.



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ISSUE	DATE	DESCRIPTION	
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PROJECT NUMBER 10123906

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**SCALE** 1" = 10'

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Texas	P.E. Firm
Registi	ation No. F-754

DATE	DESCRIPTION
01/21/2021	CONFORMED CONTRACT DRAWINGS

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01/21/2021

PROJECT NUMBER 10123906



ISSUE	DATE	DESCRIPTION
0	01/21/2021	CONFORMED CONTRACT DRAWINGS



- 1. REFERENCE CIVIL FOR SITE CONTROL POINTS AND WALL RADII, TYP ALL RETAINING WALLS.
- REFERENCE CIVIL FOR SITE GRADING, LAYDOWN CURB, AND CURB AND GUTTER DETAILS.

**SCALE** 1" = 10'

**SHEET** 68 of 350 S10-207



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0	01/21/2021	CONFORMED CONTRACT DRAWINGS	













ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL



## PLAN NOTES:

7

- 1. REFERENCE CIVIL FOR SITE CONTROL POINTS AND WALL RADII, TYP ALL RETAINING WALLS.
- 2. REFERENCE CIVIL FOR SITE GRADING, LAYDOWN CURB, AND CURB AND GUTTER DETAILS.



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Texas P.E. Firm 0 01/21/2021 CONFORMED CONTRACT DRAWINGS				
	Texas P.E. Firm		01/21/2021	CONFORMED CONTRACT DRAWINGS



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- END OF RETAINING WALL



PLAN NOTES:

- 1. REFERENCE CIVIL FOR SITE CONTROL POINTS AND WALL RADII, TYP ALL RETAINING WALLS.
- 2. REFERENCE CIVIL FOR SITE GRADING, LAYDOWN CURB, AND CURB AND GUTTER DETAILS.

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		525
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Tex	as P.E. Firm
Reg	gistration No. F-754

ISSUE	DATE	DESCRIPTION	
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FJS
Texas P.E. Firm Registration No. F-754

0	01/21/2021	CONFORMED CONTRACT DRAWINGS
ISSUE	DATE	DESCRIPTION

![](_page_19_Figure_3.jpeg)

PROJECT MANAGER	W. D. WEHNER
DESIGNED BY	LEM
DRAWN BY	HDG
CHECKED BY	CAZ
DATE	OCTOBER 2020
PROJECT NUMBER	10123906

![](_page_19_Picture_6.jpeg)

![](_page_19_Picture_7.jpeg)

ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL

![](_page_19_Figure_9.jpeg)

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![](_page_20_Figure_0.jpeg)

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Texas F	P.E. Firm
Registra	ation No. F-754

ISSUE	DATE	DESCRIPTION	
0	01/21/2021	CONFORMED CONTRACT DRAWINGS	

PROJECT MANAGER       W. D. WEHNER         DESIGNED BY       LEM         DRAWN BY       HDG         Image: state sta
DESIGNED BY LEM DRAWN BY HDG
DRAWN BY HDG
CHECKED BY CAZ
DATE OCTOBER 2020
PROJECT NUMBER 10123906

![](_page_20_Picture_5.jpeg)

![](_page_20_Picture_6.jpeg)

ULLRICH WTP LOW SERVICE PUMP

STATION	STRUCTURAL WATER QUALITY POND WALL SECTIONS AND DETAILS				
ENEVVAL	0	<u> " 2</u> "	FILENAME SCALE	S10-305.dwg AS NOTED	SHEET

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73 of 350 )-305

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![](_page_21_Figure_0.jpeg)

![](_page_21_Picture_1.jpeg)

![](_page_21_Picture_2.jpeg)

			PROJECT MANAGER	W.D. WEHNER
			DESIGNED BY	AR / FRV
			DRAWN BY	AR
1	05/18/2021	RFI030	CHECKED BY	FRV
0	01/21/2021	CONFORMED CONTRACT DRAWINGS	DATE	JANUARY 21, 2021
ISSUE	DATE	DESCRIPTION	PROJECT NUMBER	10123906
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![](_page_21_Figure_4.jpeg)

![](_page_21_Picture_5.jpeg)

ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL

![](_page_21_Figure_7.jpeg)

![](_page_21_Figure_8.jpeg)

![](_page_22_Figure_0.jpeg)

![](_page_22_Picture_1.jpeg)

![](_page_22_Picture_2.jpeg)

0	01/21/2021	CONFORMED CONTRACT DRAWINGS
ISSUE	DATE	DESCRIPTION

PROJECT MANAGER W.D. WEHNER DESIGNED BY AR / FRV DRAWN BY AR CHECKED BY FRV DATE OCTOBER 2020 PROJECT NUMBER 10123906

![](_page_22_Picture_5.jpeg)

![](_page_22_Picture_6.jpeg)

ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL

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<u>SUPERI</u>	MPOSED DESIGN LOAD PLAN NOTES				
1.	Plans show location and loads for all known large mechanical, electrical, and plumbing equipment. Any equipment not shown shall be brought to the attention of the structural engineer.				
2.	All equipment loadings shown are considered dead loads.				
3.	Actual equipment weights to be verified by the contractor. If actual weights exceed the assumed value, contact the structural engineer for possible redesign of supporting structure.				
4.	Weigh of housekeeping/inertia pads below the equipment has been included in the structural design, but are not included in the operating weight or the floor loading superimposed load values listed.				
<u>SUPERI</u>	MPOSED DESIGN LOAD PLAN LEGEND				
	TYPICAL 100 PSF (LIVE LOAD)				
	EQUIPMENT REF SCHEDULE (DEAD LOAD)				
	TRAFFIC 250 PSF (LIVE LOAD)				

EQUIPMENT SUPERIMPOSED DEAD LOAD SCHEDULE					
MARK	DESCRIPTION	OPERATING WEIGHT (LBS)	FLOOR LOAD (PSF)		
AH-1A	AIR HANDLING UNIT	1,365	45		
AH-1B	AIR HANDLING UNIT	1,365	45		
SWGR-100	12470V SWITCHGEAR	31,520	230		
SWGR-200	12470V SWITCHGEAR	31,520	230		
SWGR-300	4160V SWITCHGEAR	12,530	115		
SWGR-400	4160V SWITCHGEAR	12,530	115		
SWGR-RES	SWITCHGEAR RESERVED FOR FUTURE	N/A	230		
MCP-001	MAIN CONTROL PANEL	14,400	170		

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![](_page_22_Picture_16.jpeg)

![](_page_23_Figure_0.jpeg)

![](_page_23_Picture_1.jpeg)

![](_page_23_Picture_2.jpeg)

0	01/21/2021	CONFORMED CONTRACT DRAWINGS
ISSUE	DATE	DESCRIPTION

![](_page_23_Picture_4.jpeg)

![](_page_23_Picture_5.jpeg)

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ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL

![](_page_23_Figure_7.jpeg)

![](_page_23_Figure_8.jpeg)

![](_page_23_Figure_10.jpeg)

CMU WALL SCHEDULE					
MARK	NOMINAL WALL THICKNESS	REINFORCEMENT	MAX HEIGHT	LOAD BEARING?	
А	12"	(2) #5 @ 24" OC	28'-6"	YES	
В	8"	#5 @ 24" OC	10'-0"	NO	

![](_page_23_Figure_13.jpeg)

![](_page_24_Figure_0.jpeg)

![](_page_24_Picture_1.jpeg)

![](_page_24_Picture_2.jpeg)

		PROJECT MANAGER	W.D. WEHNER			
		DESIGNED BY	AR / FRV	TE OF TALL	OFAI	
		DRAWN BY	AR			
		-		E ROMO DE VIVAR		
						STATION
		CHECKED BY	FRV			ELECTRICAL EEED RI
0 01/21/2021	1 CONFORMED CONTRACT DRAWINGS	DATE	OCTOBER 2020	01/21/2021	WUED	
ISSUE DATE	DESCRIPTION	PROJECT NUMBER	10123906			

![](_page_24_Figure_4.jpeg)

![](_page_24_Figure_5.jpeg)

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FILENAME 18051 Ullrich\_LSPS

SHEET 77 of 350

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![](_page_25_Figure_0.jpeg)

![](_page_25_Picture_1.jpeg)

![](_page_25_Picture_2.jpeg)

0	01/21/2021	CONFORMED CONTRACT DRAWINGS	
ISSUE	DATE	DESCRIPTION	

![](_page_25_Picture_4.jpeg)

![](_page_25_Picture_5.jpeg)

![](_page_25_Picture_6.jpeg)

**ULLRICH WTP** LOW SERVICE PUMP STATION ELECTRICAL FEED RE

	ROOF FRAMING PLAN	l	
ENEWAL	0 1" 2" FILENAME SCALE	18051 Ullrich_LSPS	sheet 78 of 350 S30-105

![](_page_26_Figure_0.jpeg)

![](_page_26_Picture_1.jpeg)

![](_page_26_Picture_2.jpeg)

0	01/21/2021	CONFORMED CONTRACT DRAWINGS
ISSUE	DATE	DESCRIPTION

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PROJECT MANAGER	W.D. WEHNER
DESIGNED BY	AR / FRV
DRAWN BY	AR
CHECKED BY	FRV
DATE	OCTOBER 2020
PROJECT NUMBER	10123906
•	•

![](_page_26_Picture_6.jpeg)

![](_page_26_Picture_7.jpeg)

ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL

![](_page_26_Figure_9.jpeg)

![](_page_26_Figure_10.jpeg)

![](_page_26_Figure_11.jpeg)

В

Α

С

![](_page_27_Figure_0.jpeg)

![](_page_27_Picture_1.jpeg)

![](_page_27_Picture_2.jpeg)

0	01/21/2021	CONFORMED CONTRACT DRAWINGS
ISSUE	DATE	DESCRIPTION

PROJECT NUMBER 10123906

![](_page_28_Figure_0.jpeg)

	ENCOTECH
	ENGINEERING CONSULTANTS
TBPE Firm 1141	8500 Bluffstone Cove, Suite B-103 Austin, Texas 78759   512.338.1101

![](_page_28_Picture_2.jpeg)

)	01/21/2021	CONFORMED CONTRACT DRAWINGS
SSUE	DATE	DESCRIPTION

PROJECT MANAGER	W.D. WEHNER
DESIGNED BY	AR / FRV
DRAWN BY	AR
CHECKED BY	FRV
DATE	OCTOBER 2020
PROJECT NUMBER	10123906

![](_page_28_Picture_5.jpeg)

![](_page_28_Picture_6.jpeg)

LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL

7	7	8

<u>SLAB P</u>	SLAB PLAN NOTES					
1.	Slab opening locations shown are based on electrical backgrounds and provided for convenience. Final locations must be verified prior to forming or placing concrete. Structural is not responsible for locations of openings within slab.					
2.	Refer to 00S-000 series for Structural Notes.					
3.	Refer to 90S-200 series for Typical Concrete					

## OUTDOOR ELECTRICAL AREA

SCALE

FILENAME | 18051 Ullrich\_LSPS

SHEET 81 of 350 S30-108

R

D

С

А

![](_page_29_Figure_0.jpeg)

ENCOTECH ENGINEERING CONSULTANTS TBPE Firm8500 Bluffstone Cove, Suite B-103 1141 Austin, Texas 78759 | 512.338.1101

![](_page_29_Picture_2.jpeg)

1	05/18/2021	RF1030
0	01/21/2021	CONFORMED CONTRACT DRAWINGS
ISSUE	DATE	DESCRIPTION
1		

4	5	

/	C.J. EDGE OF CONC.							
	90° HOOKS	[						
		HOC	OK DEVE	LOPME	NT LENC	GTH SCH	IEDULE,	Ldh
		BAR SIZE	3000 psi	4000 psi	5000 psi	6000 psi	7000 psi	8000 psi
		#3	9"	8"	7"	6"	6"	6"
		#4	11"	10"	9"	8"	8"	7"
	MIN. Lan	#5	1'-2"	1'-0"	11"	10"	9"	9"
		#6	1'-5"	1'-3"	1'-1"	1'-0"	11"	11"
	180° HOOKS	#7	1'-8"	1'-5"	1'-3"	1'-2"	1'-1"	1'-0"
		#8	1'-10"	1'-7"	1'-5"	1'-4"	1'-3"	1'-2"
		#9	2'-1"	1'-10"	1'-8"	1'-6"	1'-5"	1'-4"
		#10	2'-4"	2'-0"	1'-10"	1'-8"	1'-7"	1'-6"
		#11	2'-7"	2'-3"	2'-0"	1'-10"	1'-9"	1'-7"

## NOTE:

- 1. TABULATED VALUES ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL WEIGHT CONCRETE
- FOR TABULATED BARS SIZES ONLY:
  - A. IF CONCRETE COVER PER ACI 318-11, SECTION 12.5.3(a), THEN A MODIFICATION FACTOR OF 0.7 MAY BE APPLIED BUT THE LENGTH MUST NOT BE LESS THAN 8 x db NOR 6 IN.
  - B. IF HOOK IS ENCLOSED IN TIES OR STIRRUPS PER ACI 318-11, SECTION 12.5.3(b),(c), THEN A MODIFICATION FACTOR OF 0.8 MAY BE APPLIES BUT THE LENGTH MUST NOT BE LESS THAN 8 x db NOR 6 IN.

CONT JOINT WITHIN 2'-0" OF

**CENTERLINE OF SPAN - UNO** 

ADDITIONAL STIRRUPS OR VERT

ADDITION TO DETAILED REINF

BARS EA SIDE OF JOINT IN

CONT KEY -

THIS DETAIL APPLIES TO GRADE BEAMS

DEEPER THAN 4'-0" AND ALL WALLS.

HORIZ REINF IS CONT THROUGH

CONSTRUCTION JOINT.

NOTE:

X

F. ROMO DE VIVAR 117502

. FOR EPOXY-COATED HOOKS, MULTIPLY THE TABULATED VALUES BY 1.2.

• •

PROJECT MANAGER

DESIGNED BY

DRAWN BY

CHECKED BY

PROJECT NUMBER

DATE

W.D. WEHNER

**JANUARY 21, 2021** 

AR / FRV

AR

FRV

10123906

	f'c=3000 psi		f'c=4000	
	CON	CRETE	CONCRE	
CLASS BAR SIZE	"A"	"B"	"A"	"
#3	1'-5"	1'-10"	1'-3"	1'
#4	1'-10"	2'-5"	1'-7"	2'
#5	2'-4"	3'-0"	2'-0"	2'
#6	2'-9"	3'-7"	2'-5"	3'
#7	4'-0"	5'-3"	3'-6"	4'
#8	4'-7"	6'-0"	4'-0"	5'
#9	5'-2"	6'-10"	4'-6"	5'-
#10	5'-10"	7'-8"	5'-1"	6'
#11	6'-6"	8'-6"	5'-7"	7'
<u>NOTE</u> :				

- 1. WHERE SPLICE TYPE IS NOT INDICATED, USE CLASS "B" SPLICE.
- LAP LENGTHS LISTED ABOVE APPLY UNDER THE FOLLOWING CONDITIONS: 1 BAR DIA.

A615 GR. 60.

LAP SPLICE SCHEDULE

3 TYPICAL DETAIL SCALE: NTS

HORIZ REINF IS CONT

VERT BARS OR BEAM

STIRRUPS WITHIN 3" OF JOINT

KEY WIDTH

GRADE BEAM KEY WIDTH

"W"

3 1/2"

5 1/2"

7 1/4"

9 1/4"

11 1/4"

**ULLRICH WTP** 

STATION

LOW SERVICE PUMP

WIDTH "T"

≤ 12"

12" - 16"

16" - 20"

20" - 24"

24" - 30"

THROUGH JOINT

. .

• • \

<u>SECTION - A</u>

- B. WALL AND SLAB BARS ARE SPACED AT LEAST 2 BAR DIA OC.

- C. FOR UNCOATED AND ZINC-COATED (GALVANIZED) REINFORCEMENT.

	f'c=40 CON0	000 psi CRETE	f'c=50 CONC	00 psi CRETE	f'c=60 CONC	00 psi CRETE	f'c=70 CON0	000 psi CRETE	f'c=80 CONC	00 psi CRETE
	"A"	"B"	"A"	"B"	"A"	"B"	"A"	"B"	"A"	"B"
)"	1'-3"	1'-7"	1'-1"	1'-5"	1'-0"	1'-4"	1'-0"	1'-4"	1'-0"	1'-4"
"	1'-7"	2'-1"	1'-5"	1'-10"	1'-4"	1'-8"	1'-3"	1'-7"	1'-2"	1'-6"
"	2'-0"	2'-7"	1'-10"	2'-4"	1'-8"	2'-1"	1'-6"	2'-0"	1'-5"	1'-10"
"	2'-5"	3'-1"	2'-2"	2'-9"	2'-0"	2'-7"	1'-10"	2'-4"	1'-8"	2'-2"
"	3'-6"	4'-6"	3'-1"	4'-1"	2'-10"	3'-8"	2'-8"	3'-5"	2'-6"	3'-2"
"	4'-0"	5'-2"	3'-7"	4'-7"	3'-3"	4'-3"	3'-0"	3'-11"	2'-10"	3'-8"
)"	4'-6"	5'-10"	4'-0"	5'-3"	3'-8"	4'-9"	3'-5"	4'-5"	3'-2"	4'-1"
"	5'-1"	6'-7"	4'-6"	5'-10"	4'-1"	5'-4"	3'-10"	4'-11"	3'-7"	4'-8"
"	5'-7"	7'-3"	5'-0"	6'-6"	4'-7"	5'-11"	4'-3"	5'-6"	4'-0"	5'-2"

A. BEAM AND COLUMN BARS ARE SPACED AT LEAST 1 BAR DIAMETER OC WITH CLEAR COVER NOT LESS THAN

D. FOR REINFORCEMENT THAT CONFORMS DEFORMED NEW BILLET STEEL BARS IN ACCORDANCE TO ASTM

B. FOR HORIZ TOP BARS WITH 12" OF CONCRETE CAST BELOW, MULTIPLY TABULATIONS BY 1.3.

4. WHERE A LARGER BAR LAPS A SMALLER BAR, THE SMALLER SCHEDULED LAP LENGTH APPLIES.

5. REFER TO "CONCRETE REINFORCING" SECTION OF THE STRUCTURAL NOTES FOR FURTHER INFORMATION.

FOR CMU REINFORCEMENT SPLICE LENGTH SCHEDULE, SEE CMU DETAILS.

![](_page_29_Figure_79.jpeg)

## TYPICAL CONCRETE DETAILS

ELECTRICAL FEED RENEWAL

SCALE

FILENAME | 18051 Ullrich\_LSPS

SHEET S90-201

8

D

С

![](_page_30_Figure_0.jpeg)

	ENCOTECH
	ENGINEERING CONSULTANTS
TBPE Firm 1141	8500 Bluffstone Cove, Suite B-103 Austin, Texas 78759   512.338.1101

![](_page_30_Picture_2.jpeg)

Texas P.E. Firm Registration No. F-754

0	01/21/2021	CONFORMED CONTRACT DRAWINGS
ISSUE	DATE	DESCRIPTION

![](_page_30_Figure_5.jpeg)

![](_page_30_Picture_6.jpeg)

![](_page_30_Picture_7.jpeg)

STATION ELECTRICAL FEED RENEWAL

SCALE

SHEET 83 of 350 S90-202

![](_page_31_Figure_0.jpeg)

![](_page_31_Picture_1.jpeg)

![](_page_31_Picture_2.jpeg)

0	01/21/2021	CONFORMED CONTRACT DRAWINGS
ISSUE	DATE	DESCRIPTION

CHECKED BY

PROJECT NUMBER

FRV

DATE OCTOBER 2020

10123906

	CLASS B S CENTERED	SPLICE ON SPAN					
/		GREATER OF L2/3 OR L3/3				#3 @ 24"	
	+ + +						<u> </u>
		T2 BARS					
s/ S	ARS CENTERED			TYPE 1	TYPE	2	
			B2 BARS				
					2'-0"	TOP BAR OF SAME SIZE AND	
						STIRRUPS	
			CLASS B SPLICE				
			CENTERED ON SUPPORT				
			L3	TYPE 5	ТҮРІ	6	
	-	1					
				$\frac{1}{2} \frac{\text{TYPICAL D}}{\text{SCALE: NTS}}$	ETAIL		
_			ALTERNATE DIRECTION — OF LAPS 180° (OPPOSITE				
5	,		COLUMN FACES)	X. · ~	$\cdot$		
					•		
					LAP 38 TIE BAR DIAMETERS	C	
			5 TYPICAL DETAIL	KOUND COLON		<b>6</b> $\frac{T}{50}$	
							l
						-(	
				• NOTE.	COLUMN BAR ABOVE		
					COLUMN BAR FROM BELOW		
			<u>PLAN</u>				
			LAPPED SPLICE IN CIRCULA <b>TYPICAL DETAIL</b>	IR COLUMN			olun YP
			SCALE: NTS			S	CALE
	PROJECT MANAGER	W.D. WEHNER AR / FRV	STATE OF 7AU	<u></u>	OF AT		
	DRAWN BY	AR	STALL CT		UL		
			F. ROMO DE VIV	AR		W SERVICE PUN	IΡ

01/21/2021

5

4

![](_page_31_Figure_5.jpeg)

STATION ELECTRICAL FEED RENEWAL

0 1"

FILENAME SCALE

FILENAME 18051 Ullrich\_LSPS

SHEET 84 of 350 S90-203

![](_page_32_Figure_0.jpeg)

**KE** ENCOTECH ENGINEERING CONSULTANTS

![](_page_32_Picture_2.jpeg)

	01/21/2021	CONFORMED CONTRACT DRAWINGS
SUE	DATE	DESCRIPTION

PROJECT MANAGER	W.D. WEHNER
DESIGNED BY	AR / FRV
DRAWN BY	AR
CHECKED BY	FRV
DATE	OCTOBER 2020
PROJECT NUMBER	10123906

![](_page_32_Picture_5.jpeg)

![](_page_32_Picture_6.jpeg)

![](_page_33_Figure_0.jpeg)

![](_page_33_Picture_1.jpeg)

![](_page_33_Picture_2.jpeg)

0	01/21/2021	CONFORMED CONTRACT DRAWINGS
ISSUE	DATE	DESCRIPTION

DESIGNED BY	AR / FRV	TE OF TALLU
DRAWN BY	AR	
		F. ROMO DE VIVAR
CHECKED BY	FRV	
DATE	OCTOBER 2020	- 01/81/2021
PROJECT NUMBER	10123906	
	DESIGNED BY DRAWN BY CHECKED BY DATE PROJECT NUMBER	DESIGNED BYAR / FRVDRAWN BYARCHECKED BYFRVDATEOCTOBER 2020PROJECT NUMBER10123906

![](_page_33_Picture_6.jpeg)

ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL

![](_page_33_Figure_8.jpeg)

**4 SECTION** SCALE: 1" = 1'-0"

CONCRETE FRAMING DETAILS

SCALE

FILENAME | 18051 Ullrich\_LSPS

SHEET 86 of 350 S90-211

B

А

![](_page_34_Figure_0.jpeg)

	ENCOTECH
	ENGINEERING CONSULTANTS
TBPE Firm	8500 Bluffstone Cove, Suite B-103
1141	Austin, Texas 78759   512.338.1101

![](_page_34_Picture_2.jpeg)

0	01/21/2021	CONFORMED CONTRACT DRAWINGS	
ISSUE	DATE	DESCRIPTION	

PROJECT MANAGER	W.D. WEHNER
DESIGNED BY	AR / FRV
DRAWN BY	AR
CHECKED BY	FRV
DATE	OCTOBER 2020
PROJECT NUMBER	10123906

![](_page_34_Picture_5.jpeg)

![](_page_34_Picture_6.jpeg)

ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL

SCALE

FILENAME | 18051 Ullrich\_LSPS

SHEET 87 of 350 S90-212

![](_page_35_Figure_0.jpeg)

	ENCOTECH
	ENGINEERING CONSULTANTS
TBPE Firm 1141	8500 Bluffstone Cove, Suite B-103 Austin, Texas 78759   512.338.1101

![](_page_35_Picture_2.jpeg)

)	01/21/2021	CONFORMED CONTRACT DRAWING
SSUE	DATE	DESCRIPTION

3

PROJECT MANAGER	W.D. WEHNER
DESIGNED BY	AR / FRV
DRAWN BY	AR
CHECKED BY	FRV
DATE	OCTOBER 2020
PROJECT NUMBER	10123906

![](_page_35_Picture_5.jpeg)

![](_page_35_Picture_6.jpeg)

ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RE

4	5	6

	TYPICAL STEEL DETAILS	
ENEWAL	0 1" 2" FILENAME 18051 Ullrich_LSPS	sheet 88 of 350 S90-301

![](_page_35_Picture_10.jpeg)

7

D

С

В

А

![](_page_36_Figure_0.jpeg)

![](_page_36_Picture_1.jpeg)

![](_page_36_Picture_2.jpeg)

)	01/21/2021	CONFORMED CONTRACT DRAWINGS
SSUE	DATE	DESCRIPTION

CHECKED BY FRV OCTOBER 2020 DATE PROJECT NUMBER 10123906

![](_page_36_Picture_5.jpeg)

![](_page_36_Picture_6.jpeg)

STATION ELECTRICAL FEED RENEWAL

FILENAME | 18051 Ullrich\_LSPS

SCALE

2	3

		LOAD BE	ARING LIN	ITEL SCHEDULE			
		NOMINAL	NOMINAL	REINF	ORCING	DEADING	
MARK	DIAGRAM	DEPTH	WIDTH	LONGITUDINAL	STIRRUPS	BEARING	
L1		8"	8"	(2) #5	-	8"	
L2		16"	8"	(2) #5	-	16"	VERTICAL WAL REINFORCING REF PLAN GROUT NUMBI
L3*		16"	12"	(2) #5	-	16"	COURSES PER NOMINAL DEP
L4*		32"	12"	(2) #5	-	16"	
L5*		40"	12"	(2) #5	-	16"	LINTEL SECTION
L6*		48"	12"	(3) #6	-	16"	<ol> <li>LINTELS SHALL REMAIN SHOU CONSTRUCTION ABOVE HAS 14 DAYS.</li> <li>REFER TO ARCHITECTURAL D AND LOCATION.</li> <li>VERTICAL CONTROL JOINTS S REINFORCEMENT.</li> <li>* REF 6 / S90-401 FOR BRIC</li> </ol>

![](_page_37_Figure_2.jpeg)

MASONRY SPLICE LENGTH (INCHES)							
	NOMINAL MASONRY SIZE						
BAK #	6"	8"	12"				
#4	24"	16"	12"				
#5	32"	24"	16"				
#6	60"	48"	30"				

CMU WALL ANCHORAGE

![](_page_37_Figure_4.jpeg)

![](_page_37_Figure_5.jpeg)

![](_page_37_Figure_6.jpeg)

PROJECT MANAGER	W.D. WEHNER
DESIGNED BY	AR / FRV
DRAWN BY	AR
CHECKED BY	FRV
DATE	OCTOBER 2020
PROJECT NUMBER	10123906
•	•

![](_page_37_Picture_10.jpeg)

![](_page_37_Picture_11.jpeg)

![](_page_38_Figure_0.jpeg)

	ENCOTECH
	ENGINEERING CONSULTANTS
TBPE Firm 1141	8500 Bluffstone Cove, Suite B-103 Austin, Texas 78759   512.338.1101

Texas P.E. Firm Registration No. F-754

![](_page_38_Picture_2.jpeg)

PROJECT MANAGER	W.D. WEHNER
DESIGNED BY	AR / FRV
DRAWN BY	AR
CHECKED BY	FRV
DATE	OCTOBER 2020
PROJECT NUMBER	10123906

![](_page_38_Picture_4.jpeg)

![](_page_38_Picture_5.jpeg)

ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL

![](_page_38_Picture_7.jpeg)

![](_page_38_Figure_8.jpeg)

![](_page_38_Figure_9.jpeg)

![](_page_38_Figure_13.jpeg)

![](_page_39_Figure_0.jpeg)

![](_page_39_Figure_1.jpeg)

3

## TYPICAL CONCRETE RETAINING WALL 1/2"=1'-0"

## TYPICAL CONCRETE RETAINING WALL REINFORCING 1/2"=1'-0"

-

	RETAINING WALL GLOBAL STABILITY TABLE							
	OVERTI	JRNING	SLID	ING	BEARING			
MARK	ACTUAL MIN. REQ'D.		ACTUAL	MIN. REQ'D.	ACTUAL	MIN. REQ'D.		
12RW-01-C	2.8	1.5	1.8	1.5	1.4	1.0		
12RW-02-C1	3.2	1.5	1.9	1.5	1.5	1.0		
12RW-02-C2	5.1	1.5	3.7	1.5	2.7	1.0		
12RW-03-C	4.3	1.5	2.9	1.5	1.3	1.0		
14RW-01-C1	NOTE 3	1.5	NOTE 3	1.5	1.0	1.0		
14RW-01-C2	2.1	1.5	1.5	1.5	1.2	1.0		
14RW-02-C1	2.2	1.5	1.5	1.5	1.4	1.0		
14RW-02-C2	NOTE 3	1.5	NOTE 3	1.5	1.0	1.0		
16RW-01-C1A/B	NOTE 3	1.5	NOTE 3	1.5	1.0	1.0		
16RW-01-C2A/B	NOTE 3	1.5	NOTE 3	1.5	1.0	1.0		
16RW-01-C3A/B	NOTE 3	1.5	NOTE 3	1.5	1.0	1.0		
16RW-01-C4	3.0	1.5	NOTE 3	1.9	1.5	1.0		
16RW-01-C5	2.0	1.5	1.4	1.6	1.1	1.0		
16RW-01-C6	1.6	1.5	2.6	1.5	1.1	1.0		
16RW-01-C7	1.8	1.5	1.8	1.5	1.1	1.0		

1. FACTOR OF SAFETY SHOWN ARE FOR WORST CASE WALL DESIGNS FOR THE GIVEN RETAINING WALL MARK. ACTUAL FACTORS OF SAFETY VARY DEPENDING ON GRADING. 2. REFERENCE SITE RETAINING WALL NOTES, THIS SHEET, FOR NET ALLOWABLE BEARING PRESSURES AND WALL DESIGN PARAMETERS. 3. WALLS HAVE COMBINED FOOTING. F.O.S AGAINST OVERTURNING & SLIDING > 1.5."

12RW-01 12RW-02 12RW-02 12RW-03 14RW-01 14RW-01 14RW-02 14RW-02 16RW-01-C 16RW-01-C 16RW-01-C 16RW-01 16RW-01 16RW-01 16RW-01

![](_page_39_Picture_7.jpeg)

ISSUE	DATE	DESCRIPTION	
0	01/21/2021	CONFORMED CONTRACT DRAWINGS	

![](_page_39_Picture_9.jpeg)

![](_page_39_Picture_10.jpeg)

![](_page_39_Picture_11.jpeg)

## ULLRICH WTP LOW SERVICE PUMP STATION ELECTRICAL FEED RENEWAL

							CON	CRETE RE	TAINING W	ALL SCHED	ULE					
MARK	H (MAX) (ft)	T (in)	TW (ft)	HW (ft)	TF (in)	V1	V2	H1	H2	H3	H4	H5	H6	D1	D2	NOTES
2RW-01-C	6	12	2	5	14	#5 @ 12	#6 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#6 @ 12	
2RW-02-C1	7.5	12	1.5	5.75	14	#5 @ 12	#6 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#6 @ 12	
RW-02-C2	2	12	1.5	2.5	14	#5 @ 12	#6 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#6 @ 12	
2RW-03-C	5.5	12	1	4	14	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	REFERENCE CIVIL FOR T/WALL ELEVATIONS.
IRW-01-C1	12	14	1.5	SEE NOTES	16	#5 @ 12	#7 @ 6	#5 @ 12	#5 @ 12	#7 @ 6	#7 @ 6	#7 @ 6	#7 @ 6	#5 @ 12	#7 @ 6	COMBINED FOOTING. SEE PLANS.
RW-01-C2	8.5	14	1.5	5.5	16	#5 @ 12	#6 @ 10	#5 @ 12	#5 @ 12	#5 @ 10	#5 @ 10	#5 @ 10	#5 @ 10	#5 @ 12	#6 @ 10	
RW-02-C1	8	14	1.5	5.5	16	#5 @ 12	#6 @ 10	#5 @ 12	#5 @ 12	#5 @ 10	#5 @ 10	#5 @ 10	#5 @ 10	#5 @ 12	#6 @ 10	
RW-02-C2	9.5	14	1.5	SEE NOTES	16	#5 @ 12	#7 @ 6	#5 @ 12	#5 @ 12	#7 @ 6	#7 @ 6	#7 @ 6	#7 @ 6	#5 @ 12	#7 @ 6	COMBINED FOOTING. SEE PLANS.
W-01-C1A/B	9.75	16	1.5	SEE NOTES	18	#6 @ 12	#9 @ 12	#6 @ 12	#6 @ 12	#8 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#9 @ 12	COMBINED FOOTING AT CULVERTS. SEE PLANS AND SECTIONS FOR ADDITIONAL DETAILS.
W-01-C2A/B	10	16	1.5	SEE NOTES	18	#7 @ 12	#9 @ 9	#7 @ 6	#7 @ 6	#9 @6	#7 @ 12	#7 @ 12	#9 @6	#7 @ 12	#9 @ 9	COMBINED FOOTING. TOP OF FOOTING 1'-0" BELOW BOTTOM OF ELECTRICAL MANHOLE.
W-01-C3A/B	10	16	1.5	SEE NOTES	18	#7 @ 12	#9 @ 9	#7 @ 6	#7 @ 6	#7 @ 6	#7 @ 6	#7 @ 6	#7 @ 6	#7 @ 12	#9 @ 9	"COMBINED FOOTING AT CULVERTS. SEE PLANS AND SECTIONS FOR ADDITIONAL DETAILS. PROVIDE #9 @ 5 VERT (IF) FOR 4' EACH SIDE OF CULVERT BLOCKOUT."
RW-01-C4	5.5	16	1.5	5	16	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	
RW-01-C5	5.75	16	1.5	3.75	12	#5 @ 12	#6 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#6 @ 12	
RW-01-C6	5.67	16	1.5	2.75	18	#5 @ 12	#6 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#5 @ 12	#6 @ 12	RETAINING WALL @ MANHOLE 1A.
RW-01-C7	5	16	2	2	16	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	RETAINING WALL @ MANHOLE.
						-				1						

6

WALL "B"

- OPTIONAL CJ

LAP TYP

![](_page_39_Figure_14.jpeg)

H6

- STD 90° HOOKS, H5

BOT

## TYPICAL REINFORCEMENT TRANSITION BETWEEN WALL TYPES 1/2"=1'-0"

2. REFERENCE PLANS AND WALL SCHEDULE FOR WALL CALLOUTS AND REINF REQUIREMENTS.

![](_page_39_Figure_20.jpeg)

1. DETAIL APPLIES WHERE WALLS CHANGE FROM

ONE WALL CALLOUT TO ANOTHER. PROVIDE

![](_page_39_Figure_21.jpeg)

![](_page_39_Figure_22.jpeg)

![](_page_39_Figure_23.jpeg)

![](_page_39_Figure_24.jpeg)

![](_page_39_Figure_25.jpeg)

![](_page_39_Figure_26.jpeg)

![](_page_39_Figure_27.jpeg)

![](_page_39_Figure_28.jpeg)

![](_page_39_Figure_29.jpeg)

![](_page_39_Figure_30.jpeg)

![](_page_39_Figure_31.jpeg)

![](_page_39_Figure_32.jpeg)

![](_page_39_Figure_33.jpeg)

5

WALL "A"

![](_page_39_Figure_34.jpeg)

![](_page_39_Figure_35.jpeg)

NOTES:

![](_page_39_Figure_36.jpeg)

4

H3

H

 $\sqrt{}$ 

2

-

Т

φIΞ

	7	8							
	KEY NOTES								
	(1) CONTINUOUS WALL DRAINAGE COMPOSITE S MIRAFI G100W OR APPROVED EQUAL. THE MII SHALL BE 145 GALS/MIN/SF IN ACCORDANCE COMPOSITE SYSTEM IN ACCORDANCE WITH M	YSTEM. PROVIDE MIRAFI 140N FOR FILTER FABRIC AND MNIMUM FLOW RATE FOR THE DRAINAGE COMPOSITE WITH ASTM D4491. INSTALL WALL DRAINAGE MANUFACTURER RECOMMENDATIONS.							
	2 10"x10" SQUARE OF DRAINAGE BOARD, INSTA MFR RECOMMENDATIONS AND FOLD GEOTEX	10"x10" SQUARE OF DRAINAGE BOARD, INSTALL WITH PLASTIC FACE TOWARD WALL, SECURE PER MFR RECOMMENDATIONS AND FOLD GEOTEXTILE AROUND EDGES TO SEAL.							
	3 CONTRACTOR IS RESPONSIBLE TO CLASSIFY CLASSIFICATION.	SOIL TYPE AND PROVIDE SAFE SOIL SLOPE BASED ON							
	$\langle 4 \rangle$ APPROVED, UNDISTURBED SUBGRADE OR ST	RUCTURAL FILL							
	$\left< \frac{1}{5} \right>$ SELECT FILL, SEE SPECIFICATIONS.								
- VERT AND	6 TEMPORARY EARTH RESTRAINT AS REQUIRE CONTRACTOR IN ACCORDANCE WITH THE CC	D AND PER CONTRACTOR'S OPTION, DESIGNED BY INTRACT DOCUMENTS.							
HORIZ REINF PER SCHEDULE,	$\langle 7 \rangle$ PROVIDE STD HOOKS (180 DEGREE AS REQD BY FOOTING THICKNESS) FOR H3 AND H6 REINF.								
TTP	SITE RETAINING WALL NOTES:								
	<ol> <li>SEE STANDARDS DRAWINGS FOR GENERAL STI</li> <li>SEE SITE-CIVIL PLANS FOR GRADING INFORMAT</li> <li>SEE CIVIL DRAWINGS FOR ALL EXTERIOR PAVIN</li> <li>WALL PARTIAL CONTROL JOINTS SHALL BE PROEXPANSION JOINTS SHALL BE PROVIDED AT 90 CONTRACT DOCUMENTS.</li> <li>FOOTINGS FOR RETAINING WALLS SHALL BE PL 3/S90-513.</li> <li>NET ALLOWABLE BEARING PRESSURE: 2750 PSI FOR CASE 2 SHOWN IN DETAIL 3/S90-513 SUBGE IN ACCORDANCE WITH THE SPECIFICATIONS.</li> <li>RETAINING WALL DESIGN PARAMETERS:</li> </ol>	STRUCTURAL NOTES AND STANDARD DETAILS. VATION AND WALL ALIGNMENT. VING AND FLATWORK. PROVIDED EVERY 30 FT (MAX). ONE (1) INCH WIDE 90 FT (MAX) SPACING IN ACCORDANCE WITH THE E PLACED ON PREPARED SUBGRADES PER DETAIL PSF FOR CASE 1 SHOWN IN DETAIL 3/S90-513. 1750 PSF BGRADE TESTING FOR FOOTINGS SHALL BE PERFORMED 3. 135 PCF.							
	A. FILL/BACKFILL UNIT WEIGHT:	135 PCF.							
	AT-REST EFP:	60 PCF							
	ACTIVE EFP:	40 PCF							
	C. LOWER 1/3 HEIGHT								
	AT-REST EFP:	90 PCF							
<del>(</del> - )	ACTIVE EFP:	80 PCF							
$\smile$	D. PASSIVE PRESSURE :	250 PCF (NEGLECT 12" OVER TOE FOR SLIDING RESISTANCE)							
	E. SURCHARGE ON HEEL SIDE OF WALLS:	270 PSF VERTICAL SURCHARGE FOR ROADWAYS							

0.50

F. SOIL/FOOTING FRICTION:

# STRUCTURAL RETAINING WALL STANDARD DETAILS

FILENAME S90-511 SCALE AS NOTED Α

![](_page_40_Figure_0.jpeg)

SUE	DATE	DESCRIPTION
0	01/21/2021	CONFORMED CONTRACT DRAWINGS

	f'c = 4.0 - 5.0 ksi						
		fy = 0	60.0 ksi				
	LAP LEN	GTH (IN.)	DEVELOPMENT I	_ENGTH L <sub>d</sub> (IN.)			
BAR SIZE	TOP	OTHER	TOP	OTHER			
#3	16	16	12	12			
#4	19	16	15	12			
#5	24	20	19	15			
#6	29	23	22	18			
#7	42	33	33	25			
#8	48	38	37	29			
#9	60	47	46	36			
#10	74	57	57	44			
#11	89	69	68	53			

![](_page_41_Figure_0.jpeg)

![](_page_41_Picture_1.jpeg)

			PROJECT MANAGER	W. D. WEHNER
			DESIGNED BY	LEM
			DRAWN BY	HDG
			CHECKED BY	CAZ
0	01/21/2021	CONFORMED CONTRACT DRAWINGS	DATE	OCTOBER 2020
ISSUE	DATE	DESCRIPTION	PROJECT NUMBER	10123906

![](_page_41_Picture_4.jpeg)

![](_page_41_Picture_6.jpeg)

![](_page_41_Picture_10.jpeg)

## STRUCTURAL **RETAINING WALL** STANDARD DETAILS

FILENAME S90-512.dwg SCALE AS NOTED

![](_page_41_Picture_13.jpeg)

![](_page_42_Figure_0.jpeg)

FJS
Texas P.E. Firm Registration No. F-754

SSUE	DATE	DESCRIPTION	
0	01/21/2021	CONFORMED CONTRACT DRAWINGS	

PROJECT MANAGER	W. D. WEHNER
DESIGNED BY	LEM
DRAWN BY	HDG
CHECKED BY	CAZ
DATE	OCTOBER 2020
PROJECT NUMBER	10123906

![](_page_42_Picture_4.jpeg)

![](_page_42_Picture_5.jpeg)

![](_page_43_Figure_0.jpeg)

![](_page_43_Picture_1.jpeg)

SSUE	DATE	DESCRIPTION
0	01/21/2021	CONFORMED CONTRACT DRAWINGS

![](_page_43_Figure_3.jpeg)

 $\sim$ 

ō

3

![](_page_43_Figure_4.jpeg)

6

3

![](_page_43_Figure_5.jpeg)

cover over h wingwalls or

6 As an aid in longitudinal approval of furnished at

- 🕖 Top longitudi 3" plus or m
- 8 Bend or cut
- 9 No longitudin

10 Space U(#4) length is les U(#4) bars a is 6'-0" and

![](_page_43_Figure_12.jpeg)

1'-0"

— Face of Rail

- S1(#4)

1 1/2"

(Тур)

1" R or Chamfer

(Typ) -

R(#4)

1 ½"

(Typ)

PROJECT MANAGER	W. D. WEHNER
DESIGNED BY	LEM
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CHECKED BY	CAZ
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	•

1'-0"

1 1/2"

(Typ)

51

53

53(#4)

0'0'•

1 1/2"

(Typ)

— Face of Rail

- S2(#4)

![](_page_43_Picture_14.jpeg)

![](_page_43_Picture_15.jpeg)

**ULLRICH WTP** LOW SERVICE PUMP S ELECTRICAL FEED RENEWAL

	7			
for structures	with overlay.			
ertical reinford horizontal reinf r retaining wall	cing has closer clear <sup>f</sup> orcing in abutment Is on traffic side of wall.			
n supporting rei bars may be us the Engineer. t the Contractor	inforcement, additional sed in the slab with the Such bars will be rs expense.			[
linal slab bar m ninus to tie rein	nay be adjusted laterally nforcing.			
as required to	o clear drain slots.			
nal wires may	be in top center of cage.			
) bars at 4" Max ss than 6'-0" to at 6" Max when greater to side	x when end region of panel side slot drain. Space end region of panel length slot drain.			
<b>TRUCTION N</b> cailing may be of Engineer, with of control for bot to provide bra can be perfor e and the ancho ars at any loca additional ancho of rail and par se shown in the fer all exposed	<b>IOTES:</b> constructed with slip-forms whe equipment approved by the En- h line and grade must be provising for slip-form operations med at a minimum spacing of 2 orage. It is permissible to we ation on the cage. If increase horage devices must be added the upper two thirds of the ca apet must be vertical transver e plans or approved by the En concrete corners.	hen approved gineer. vided. Tack is acceptable. 3 ft between eld to U, WU d bracing is and welding age. sely unless gineer.		(
RIAL NOTES: de Class "C" cor	; ncrete. Provide Class "C" (HPC	C) if required		
ere. de Grade 60 rei oated or galvar oated or galvar med Welded Wi ize and spacing noted otherwise ited for Bars R required for r de bar laps, wh	inforcing steel. mize all reinforcing steel if sla nized. re Reinforcement (WWR) (ASTM g may be substituted for Bars e. Deformed WWR (ASTM 1064 Chand S, as shown. Provide the reinforcing bars. mere required, as follows: Uncoated or galvanized	ab bars are A1064) of U and WU ) may be be same ~ #4 = 1'-7"		
	Epoxy coated	$\sim #4 = 2'-5''$		
<b>RAL NOTES:</b> rail has been set to meet MAS of 50 mph and on is used. Wh nis rail can only t use this railing more than 5" onchorage details ation for select ere in plans for drawings are n ge weight of ra	Successfully evaluated by full- SH TL-4 criteria. This rail ca greater when a TL-3 rated gu nen a TL-2 rated guard fence of y be used for speeds of 45 m ng on bridges with expansion ' movement. Is shown on this standard may structure types. See approp these modifications. not required for this rail. ailing with no overlay is 413 p	-scale on be used for uard fence transition is ph and less. joints v require priate details		E
dimensions are	e clear dimensions, unless not	ed otherwise.		
orcing bar dime	nsions shown are out-to-out o	of bar.		
	SHEET 2 OF 2			
	SHELT 2 OF 2	Bridge		
Texas Dep	partment of Transportation	Division Standard		
	<b>·</b>			
	TDAEEIC DAIL			
	I KAFFIC KAIL			
				F
	TYPE T222	2		
E: rlstd003-18.dg	n DN: TxDOT CK: TxDOT DW	I: JTR CK: TXDOT		
REVISIONS				
	DIST COUNTY	SHEET NO.		
				]
		STRUC		
	חצד	κειαινί Οτ στανγ	NG WALL	S
STATION		• 17 M B		-

FILENAME S90-515.dwg SCALE AS NOTED

**SHEET** 96 of 350 S90-515